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SPECIAL STUDY S-52-2

EMS RIVER

ARTIFICIAL FLOODING

POTENTIALITIES

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Prepared by
Military Hydrology R & D Branch
Engineering Division
Washington District Corps of Engineers
Washington, D. C.
August 1952

SPECIAL STUDY S-52-2

ES RIVER

ARTIFICIAL FLOODING POTENTIALITIES

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EMS RIVER
ARTIFICIAL FLOODING POTENTIALITIES

SECTION I

INTRODUCTION

1-01 ASSIGNMENT.

This special study was assigned to the Military Hydrology Research and Development Branch, Engineering Division, Washington District, by letter from Office, Chief of Engineers, ENOWE, to the Division Engineer, North Atlantic Division; subject, "Military Hydrology R & D Project No. 8-72-12-001; Special Assignments," dated 27 May 1952.

1-02 PURPOSE AND SCOPE.

a. This report presents information regarding the hydraulic effects and nature of artificial flooding potentialities in the Ems River basin. It consists largely of a compilation and consolidation of information presented in various intelligence documents and technical publications, with certain supplementary analyses and discussions. Additional studies are needed to adequately cover the subject for general military requirements.

b. The report is designed to furnish basic data and results of analyses needed to answer questions concerning:

(1) Normal and extreme stages and surface velocity including duration, at key stations on the Ems River.

(2) Stream characteristics including gradients, depths and widths of channel and flood plain in various reaches of the waterways of the basin.

(3) Data concerning locations and zero elevations of gaging (pegel) stations.

(4) Data concerning location and dimensions of navigation structures, levees and bridges on the waterways of the Ems River basin.

(5) The extent of flooding created by erection of temporary dams on the Ems River.

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(6) The magnitude and duration of flood waves and flow variations created by destruction or operation of stress control structures and the effect on bridging, crossing, and navigation of the Ems River.

1-03 ARRANGEMENT:

This report is subdivided as follows:

Section I Introduction
Section II Drainage Basin Characteristics and Developments
Section III Hydrologic Characteristics
Section IV Artificial Flooding Potentialities
Section V Effect on Military Operations

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~~Tables~~
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Exhibit A Description of Bridges and Dams, Ems River
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1-04 DEFINITION OF TERMS.

a. Equivalent English-Metric Terms. Both the English and metric systems are used in this report. Conversion factors are presented for reference in Table 1.

b. Hydrologic Terms and Abbreviations. The following abbreviations are used in this report: l for liters, m for meters, km for kilometers, m³ for cubic meters, m/s for meters per second, m³/s for cubic meters per second. Abbreviations applicable to stage and discharge are defined in Table 2.

1-05 REFERENCES.

All references cited in this report are listed in the bibliography following Section V of the text.

SECTION II

DRAINAGE BASIN CHARACTERISTICS AND DEVELOPMENTS

2-01 GENERAL.

a. The Ems River is the westernmost of the large rivers in Germany. It is located in the provinces of Hannover and Westfalen in the northwest part of Germany near the Holland border. The river flows northward to Emden emptying into Dollart Bay, an estuary of the North Sea. The Ems River forms the basis of a large canal system, including the Dortmund-Ems Canal which follows the Ems valley for much of its course. A general map is shown on Plate 1 and a more detailed location plan is included on Plate 2. Detailed descriptions are contained in Exhibits A and B and in References 1 and 2 listed in the bibliography of this report.

b. This report is confined to consideration of the main stem of the Ems River, the Dortmund-Ems canal, and the canals on the left side of the Ems River east of the Germany-Holland border.

2-02 TOPOGRAPHY.

The general topography of the Ems River basin is illustrated on the Physiographic Diagram shown as Plate 3. The Ems River rises on the southwestern slope of the Teutoburger Wald at an altitude of about 130 m and flows westward across the shallow land depression of the "Münster Bay," emerging onto the North German Plain through a steep, shallow gorge near Rheine. There it continues northward in a sandy valley between moorlands. The lower reaches cross flat low-lying reclaimed marshlands and enter the broad, shallow Dollart Bay, a wide estuary of the North Sea. The divide between the Ems and Lippe Rivers is indeterminate and is crossed by the Dortmund-Ems Canal. Detailed topographic descriptions appear in the documents listed as References 1 and 3 in the Bibliography.

2-03 GEOLOGY.

The Ems River meanders between low hills of "Geest," (heath and moorland) through an alluvial valley 1 to 5 km wide. The river-bed consists almost entirely of sand, with outcrops of chalk and chalk-marl near the Rhine gorge and with patches of clay in the tidal reaches. Detailed descriptions of geologic conditions are contained in References 1 and 3.

2-04 DRAINAGE AREAS.

The drainage area of the Ems River and its tributaries is 12,482 km², the smallest drainage basin of the major German rivers. For comparison, the drainage area of the Weser River is 45,548 km².

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Drainage areas at key gaging stations on the Ems River are included in Tables 3 and 4. The areas drained by the Ems River and its major tributaries are as follows:

<u>River</u>	<u>Location</u>	<u>Drainage area km²</u>
Werre	Ems confluence	765
Aa	" "	933
Hase	" "	3126
Leda	" "	2203
Ems	Werre confluence	1870
"	Aa "	3871
"	Hase "	5079
"	Leda "	9499
"	Mouth	12482

2-05 GRADIENTS AND PROFILES.

a. Gradients are indicated on the profiles shown on Plates 4-10, inclusive. Following are listed average gradients of the Ems River:

<u>Reach</u>	<u>River Km</u>	<u>Avg. Gradient per 10,000</u>
Source - Beiwinkels Mill	430-414	32
Beiwinkels Mill - Werre River	414-311	4.5
Werre River - Rhine	311-251	1.6
Rhine - Bentlage	251-246	9.6
Bentlage - Meppen	246-176	2.2
Meppen - Leda River	176- 78	1.6
Leda River - Mouth	78- 0	0.7

b. Elevations in this report are in meters above "Normal Null" (N.N.), the zero of the German land survey datum, corresponding to mean sea level on the Baltic coast and slightly below mean sea level on the German North Sea Coast. In certain Dutch and other publications, elevations may be referenced to "Wamthourigheids Amsterdamsch Peil" (W.A.P.), the modern Dutch survey datum. The zero elevation of the German N.N. datum lies 0.012 m above the zero of the Dutch N.A.P. datum.

c. River distances along the Ems River are expressed in this report as kilometers above Vorkum Island, the mouth of the Ems River in Dollart Bay. In Exhibit B, distance is measured in a downstream direction, and three different kilometrage zero points are employed for different reaches: at the source, at Greven, and at Meppen. The systems are indicated on the General Map, Plate 1. In certain other publications, such as Reference 2, distances are measured upstream or downstream, from critical navigation locations. The conversion factors are variable as the bases of measurement are different.

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d. Distances along the Dortmund-Ems Canal are measured in kilometers northward from Dortmund and are so shown on the Location Plan, Plate 2 and on the Profile, Plate 7. Kilometrage of other canals are measured from their junction with main canal intersections as indicated on the profiles of Plates 7 to 10.

2-06 CHANNEL DEPTHS.

a. The depth of the Ems River is shallow and variable in some upriver locations but fairly deep in the pools above the dams. Detailed depth data are contained in Exhibit A, on Plates 4 to 6, and in References 1 and 2. A tabulation of representative average depths along the Ems River follows:

<u>Reach</u>	<u>River km</u>	<u>Avg. Depth at HW(m)</u>
Source -- Werse River	430-311	0.6 (2-4 m at dams)
Werse River -- Rhine	311-251	0.9-4
Rhine -- Meppen	251-176	1.5-4
Meppen -- Leda River	176- 78	1.2-4
Leda River -- Emden	78- 51	5.5-6.5
Emden -- Mouth	51- 0	10

b. The depth of the Dortmund-Ems Canal was originally 2.5 m but is being increased to 3.5 m, the standard depth for major German navigation canals. Minor navigation canals, such as those on the left side of the Ems River, are 1.5 to 2 m deep. Depths are indicated on the canal profiles, Plates 7 to 10 and in Appendixes A and B.

2-07 CHANNEL AND FLOOD PLAIN WIDTHS.

a. Widths of channel along the Ems River are shown in Exhibit A. The width of valley subject to flooding can be estimated by reference to the General Map, Plate 1. Following is a general indication of the channel and flood plain widths:

<u>Reach</u>	<u>River km</u>	<u>Channel width (m)</u>	<u>Flood Plain Width (km)</u>
Source -- Werse River	430-311	7.5-25	0.5-1.5
Werse River -- Rhine	311-251	20-30	0.5-1.5
Rhine -- Meppen	251-176	40-50	0.5-3
Meppen -- Leda River	176- 78	50-300	1.5-7
Leda River -- Emden	78- 51	550	10-15

b. The Dortmund-Ems Canal has a surface width of 30 to 40 meters and the navigation canals on the left side of the Ems are 13 to 16 m wide. Detailed data upon canal widths are contained in References 1 to 5.

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2-06 NAVIGATION.

a. The Ems River forms the basis of an extensive navigation canal system. The Dortmund-Ems Canal follows the Ems Valley from Dortmund to Emden, and in part is a unit of the Mittelland Canal system which extends across Germany from the Rhine River to the Elbe River, as is shown on Plates 1 and 10. The Emsen Canal links the Dortmund-Ems Canal and the Weser River estuary. Description of the most important navigable waterways is contained in paragraph 2-12 and in Exhibit B; locations are shown on plates 1 and 2 and profiles are presented as Plates 4-10. Additional information regarding navigation may be found in the documents listed as References 1-4 in the Bibliography.

b. The Upper Ems River can be navigated in the reach from the Schoneflieth Dam (km 296) to Harenknecht Dam (km 214) by the so-called "Ems-Ponten," which are 26 m long by 5.3 m wide, and which are normally loaded to 80 tons of their full 150 ton capacity so as not to exceed a 0.8 m draft. Traffic in that section is insignificant. The Middle Ems from Harenknecht Dam to the Hase River confluence at Heppen (km 176) contains so many bends that it now has been practically abandoned for navigation in favor of the adjacent reach of the Dortmund-Ems Canal. The lower Ems from Heppen to Papenburg (km 92) has been canalized as part of the Dortmund-Ems Canal, and is maintained to a minimum depth of 2 m by dredging in order to permit passage of 1000 ton barges. Papenburg is accessible to sea-going vessels of 3.5 m draft. The channel project depth at Leer (km 77) is 4.1 m, and between that port and Emden is 3.5 m. A lateral canal parallels the Ems River between Oldersum (km 62) and Emden (km 51) and is used by river boats during heavy weather to avoid dangerous conditions on the Ems River. Detailed navigation data are contained in Reference 2.

c. The Ems River below Emden rarely freezes over, but drift ice forms during severe winters. The reaches of the Ems River above Emden sometimes freeze with ice up to 30 cm thick, but interruption of navigation due to ice is an exceptional occurrence. Navigation on the smaller canals is suspended for periods of 2 to 3.5 months during severe winters. Navigation on the Ems River proper is interrupted for an average of 27 days annually due to ice or floods. Following is a tabulation of icing conditions on the Ems River at Lingen and Mesum:

	<u>Lingen</u> (km 208)	<u>Mesum</u> (km 200)
Mean number of days with ice	10.6	17.4
Maximum number of days with ice	34	37
Extreme occurrence of first ice	27 Nov.	20 Nov.
Extreme occurrence of last ice	3 Mar.	26 Feb.

Metric tons

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2-09 REGULATION.

Stages and flow on the Ems River may be regulated by operation of the locks and dams for navigation, power generation, and irrigation. Limited diversions of water between the Ems River, Dortmund-Ems Canal, and the other canals is accomplished by pumping and by operation of control structures. Detailed description of flow regulation on the Ems River and adjacent canals is included in Exhibit B and in References 1 and 2. Locations of structures are shown on the maps and profiles, Plates 1 to 10, inclusive.

2-10 DAMS AND RESERVOIRS.

a. Reservoirs. No high dams or large reservoirs exist in the Ems River basin, except for a 9 million m³ storage reservoir in the Soeste river valley near Freisoythe, south of the Kuesten Canal.

b. Navigation Dams. Detailed description of the numerous navigation locks and dams located on the various streams and canals of the Ems River basin is contained in Exhibits A and B, and locations are shown on the maps, Plates 1 and 3, and on the profiles, Plates 4 to 10.

c. Mill Dams. On the Ems River upstream from Rhine (km 251), there are located 15 small mill dams of minor importance. Descriptive data appears in Exhibit A and locations of the most important structures are shown on the profiles, Plate 4.

2-11 LEVEES.

No extensive levee system exists in the non-tidal reaches of the Ems River; a number of short local levees built to about 2 to 3 m above the channel bed suffice to protect adjacent farmland from summer floods. In the tidal reaches, a complete system of levees, canals, and pump stations is maintained to prevent re-flooding of the large areas of low-lying reclaimed land along the river. The main levees in that region are 4 to 5 m high.

2-12 CANALS.

a. The Dortmund-Ems Canal provides a part of the connecting link for barge traffic between the Rhine River and the North Sea and is an important part of the Mittelland Canal system. Enlargement of the canal to a project depth of 3.5 m has been undertaken to permit use of 1500-ton boats. Work was started about 1930 and was expected to be finished by 1942, but was not complete in 1945, the date of latest information. Location plan, profile, and typical cross-sections of the Dortmund-Ems Canal are shown on Plates 2, 7, and 11, respectively. Detailed navigation data are contained in References 1 through 9.

Metric tons

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b. The Ems-Seiten Canal is a lateral navigation canal by-passing the Ems River and Dortmund-Ems Canal from Hesse to Papenburg. Construction was started prior to the war, but abandoned. No information is available on the current status of construction. The proposed dimensions of the canal are 40 m wide and 3.5 m deep, sufficient for 1500 ton barges. Location of the proposed canal is shown on Plate 2, and the planned profile on Plate 7.

c. The canals on the left side of the Ems River provide for barge traffic between the German and Holland canal systems. Barges used are 27 m long by 5.8 m wide and normally carry 30 tons, but can be loaded to 150 tons for travel on the Ems-Vechte Canal. The Vechte River traffic is restricted to 20 ton capacity boats. References 1 and 2 contain detailed information. See Plates 1 and 2 for locations, Plates 8 and 9 for profiles, and Appendix B for additional description.

2-13 BRIDGES.

Locations and clearances (wherever data are available) of major bridges across the Ems River are shown on the profiles of Plates 4 to 6, inclusive. Tabulations of pertinent bridge data are included as Appendix A and examined in References 1, 2, and 10 of the Bibliography. Reliable information upon post-war reconstruction subsequent to 1945 is not available.

metric tons

SECTION III

HYDROLOGIC CHARACTERISTICS

3-01 GENERAL.

Information regarding Ems River stages and discharges, stage duration and seasonal variation and current velocities are herein presented in generalized graphical form insofar as practicable to facilitate application of the data to specific military problems. References cited should be utilized for supplementary data.

3-02 CLIMATOLOGY.

Climatological data for the region covered by this report may be found in References 2 and 3. The annual rainfall in North Germany decreases from about 700 mm on the North Sea Coast to about 600 mm at Hanover and to about 500 mm at Posen and increases about 10 mm for each 100 meters of altitude. The maximum rainfall occurs in summer, but due to greater summer-time infiltration and evaporation losses the rate of runoff is greatest during the winter months.

3-03 STREAM GAGING STATIONS.

A number of gages have been established on the Ems River and its tributaries and records are published for the more important stations. Locations of gages of primary importance are shown on the General Map, Plate 1 and on the stream profiles, Plates 4 to 6. Statistical records for the gaging stations are published in the German Hydrologic Yearbook, References 11 and 12.

3-04 RIVER STAGES.

a. Records. Data regarding the maximum, mean, and minimum stages of record at key gaging stations on the Ems River are presented in Tables 3 and 4, together with other pertinent data. Mean daily stages and other gage data may be found in References 11 and 12.

b. Seasonal Variation. The range of stages for each month of the year is indicated in Tables 3 and 4. It may be observed that the average stages for stations in non-tidal reaches during the winter, November through April, are consistently higher than the corresponding average stages during the summer months, May through October. The maximum MHW, MW, and MHW occur in January or February and the minimum in July through September. The occurrence of higher stages during winter can also be observed on the Stage Duration Curves of Plate 12.

c. Stage Duration. Stage duration curves for stations on the Ems River, showing the percent of time a given stage was equalled or exceeded during the period of record, are presented on Plates 12 and 13. It may be observed that the mean stage (MW) shown on Tables

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3 and 4, (representing an arithmetical average) differs from the median stage, that stage, shown on Plates 12 and 13, which has been equalled or exceeded 50% of the time. That difference is insignificant for stations in the tidal reaches, but is appreciable for some non-tidal stations. The following table of yearly mean and median stages shows the difference for key stations.

Station	Mean Stage (cm) (m)	Median Stage (cm) (50% of time)
	(Tables 3 & 4)	(Plates 12 & 13)
Oreven	243	210
Rhaine	226	190
Versen	152	150
Papenburg (High-tide)	641	650
(Low-tide)	478	478
Emden (High-tide)	628	630
(Low-tide)	327	320

d. Tide Variations. The tidal range between mean high tide (MHTw) and mean low tide (MLTw), as indicated in Tables 3 and 4, varies from about 3 m. at Emden to about 1.6 m at the upstream gage at Papenburg. It may be observed from those tables and also from the profile of Plate 6, that the high high-tide stages (HHHTw, HHTw, and MHTw) are higher at the gages nearer the sea than at those farther upstream. In the case of the low-tide stages a converse situation is evidenced. The stage duration curves of Plate 13 present a graphical indication of the tidal variation.

3-C5 RIVER DISCHARGE.

a. Discharge Records. Mean daily discharge, monthly and annual mean and extreme discharges are contained in the document listed as Reference 11 in the Bibliography. A tabulation of statistical discharge data at key stations on the Ems River is presented in Table 5. The duration of discharge would be practically identical with the trends evidenced by the stage duration curves of Plates 13 and 14 and described in paragraph 3-04c. The expected approximate duration of a given flow can be determined by obtaining the corresponding stage from the discharge-stage relation curves of Plate 14 to enter the duration curves of Plate 12. Median flows equalled or exceeded 50 percent of the time, established by application of the stage-discharge relation of Plate 14 to the stage duration curve of Plate 12 are tabulated below:

Station	River km	Median Flow (m ³ /s)		
		Winter	Summer	Year
Oreven	295	25	7	15
Rhaine	250	30	9	18
Versen	158	60	20	35

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b. Stage-discharge Relation. Average stage-discharge relation curves for 3 key stations on the Rar River are presented on Plate 14. These curves were estimated from discharge measurements and from equivalent stage-discharge data contained in References 11 and 12 of the Bibliography.

3-06 RIVER VELOCITIES.

a. General. The velocity of stream flow varies according to the conformation of the river bed, the depth, obstructions and restrictions, variation in slope, etc. Channel improvements and cut-offs, training walls and levees, operation of dams and other modifications of natural conditions appreciably affect the stream velocity. Influent rivers in flood tend to elevate the main river waters at the point of confluence according to the magnitude of the flood, thus tending to reduce the main river slope above and to increase it below the confluence point. Accordingly, correlations between river stages and surface velocities at gaging stations cannot be interpreted as applicable to all points along the adjacent river sections, but only serve as general indications.

b. Surface Velocities.

(1) Insufficient basic information concerning stream hydraulic functions (cross-sectional area, wetted perimeter, slope of water surface, roughness factor) is available to permit accurate determination of stream velocities. Estimates were based upon velocities observed during discharge measurements at gaging stations and recorded in Reference 12. Velocities so obtained are not necessarily indicative of the adjacent stream reaches for reasons discussed in paragraph 3-06a, and probably tend to be higher than that for the adjacent stream reaches because discharge measurements are normally made at locations of restricted sections in order to facilitate measurement. The deduced mean velocities were multiplied by 1.18 (i.e. $1/0.85$) to obtain corresponding surface velocities. A tabulation of estimated surface velocities at key locations follows:

Station	River km	Estimated Mean - Surface Velocity (km/hr)			
		HHW	LHW	LW	LWN
Rheda	360	-	3.5	2.0	1.5
Oreven	295	2.8	2.7	2.5	2.3
Rheine (in gorge)	250	4.8	4.5	2.5	0.8
Hanckenfaehr	214	-	3.0	1.7	-

(2) In order to give an indication of the relative frequency of velocities of various magnitudes, the surface velocities corresponding approximately to various stages of key gaging stations

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3-06

along the Rhine River are shown on the stage duration curves of Plate 12 opposite the corresponding gage heights.

c. Flood Wave Travel Time. Examination of available flood crest times as indicated by noon-day readings recorded in the official German Hydrological Yearbook, References 11 and 12, for the floods of November 1890, November 1926, November 1930, February 1937, and January 1938, indicates an average rate of progression of cresting time of natural floods as follows:

<u>Reach</u>	<u>River km</u>	<u>Average Peak Travel Rate (km/hr)</u>
Creven-Rhine	295-250	1.9
Rhine-Hanckenfaehr	250-214	1.4
Hanckenfaehr-Jersen	214-168	1.2

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SECTION IV

ARTIFICIAL FLOODING POTENTIALITIES

4-01 GENERAL

a. The term "artificial flood" as used in this report applies to any major increase in the extent of flooding, over that normally prevailing with existing developments, that is brought about by manipulation of control structures, breaching of dams or levees, or temporary damming operations designed to create flooding conditions. Applications of artificial flooding considered in this report fall into the following four general categories:

(1) Still-water barriers, created by flooding land to form water obstacles, using such means as breaching levees, diverting flow from canals, raising crests of existing dams or constructing temporary dams.

(2) Drainage obstacles or mud-flats, in which the wetness of the soil is increased to form muddy or marshy conditions that would impede military traffic, brought about by disrupting the normal drainage of land, destroying pumping and drainage facilities used to drain marshy or low land, or by inducing shallow inundation of flood plains or reclaimed land. Mudflats may also be formed by draining areas normally inundated by reservoirs or ponds.

(3) Streamflow variations, in which changes in discharges, depths, velocities and widths of streams are brought about to hinder stream-crossing operations or navigation such as might be accomplished by opening and closing outlet works of water control structures.

(4) Major flood waves, created by sudden breaching of a dam to release large quantities of impounded water.

b. Many opportunities exist for effective use of "still-water barriers" and "drainage obstacles" in certain areas of the Ems River basin. The possibilities of such flooding is reviewed in the following paragraphs. Opportunities also exist for effective use of "stream flow variation" and "major flood waves" and should be fully considered in the planning of military operations. This report deals principally with creation of "still water barriers" and "drainage obstacles" along the course of the Ems River; however, certain qualitative evaluation of the possibilities of utilization of "stream flow variation" and "major flood waves" is included.

c. A considerable amount of information regarding artificial flooding possibilities in the Ems River basin is contained in the study

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made for the German General Staff, a translation of which is included as Exhibit B of this report, and which has been utilized in preparation of the analyses of artificial flooding potentialities, reviewed in the following paragraphs. Examination of various other documents in connection with preparation of this report disclosed evidence that some studies had been made by Dutch, American, and British sources; however, those references were not available at the time of writing of this report, but are listed in the Bibliography as References 13 to 15, inclusive.

4-02 STILL-WATER BARRIERS AND DRAINAGE OBSTACLES.

a. General. The studies reviewed in this paragraph pertain to the artificial flooding effects that might be produced by creation of still-water barriers and drainage obstacles along the Ems River. The studies were largely based upon a map study of the area, utilizing the 1:25,000 GSGS 4414 map series supplemented by data from References 1 and 3. Exact determination of elevations, contour lines, and flooding boundaries from those maps was difficult; however, the results contained in this report are believed to offer a good indication of the relative possibilities of such flooding. First-hand information should be obtained by local reconnaissance regarding the location and dimensions of levees, roadfills, and culverts and the ground elevations in the vicinity of specific locations in order to accurately establish the area subject to artificial flooding.

b. Hydrologic Considerations. The effectiveness of artificial flooding is contingent in large measure upon the natural hydrologic conditions prevailing at the time of the operation. The amount of water stored and available within the basin, the stage and rate of flow in the streams, are all important factors. Reference is made to Section III of this report for detailed description and to following sub-paragraphs for summarization of those pertinent hydrologic considerations.

(1) The mean discharge of the Ems River during the period 1926 to 1935 ranged from 18.5 m³/s at Greven (km 295) to 68 m³/s at Versen (km 168), averaging about 30 m³/s for the reaches of primary interest. During the dry summer months the average minimum flow would be as low as 3 m³/s. Flood flows as high as 460 m³/s have been recorded at Rheine (km 250). Reference is made to Table 5 for additional discharge data.

(2) The natural water supply of the Ems River basin is obtained from rainfall over the drainage areas and supplemented by water supplied to the canals from sources outside of the Ems drainage area. In order to conserve the limited supply, "thrift basins" and repumping of lockage discharge to the higher levels is resorted to at many of the locks. Water is withdrawn from the canal system for water supply of the city of Munster and for other uses at an average rate of approximately 0.3 m³/s. The water in the canal system can be

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replenished by pumping of water from the following external sources:

<u>Source</u>	<u>Pump Capacity</u> <u>m³/s</u>
Lippe River	20
Rhine River	13
Waser River	10

However, during periods of extended drought, only the water supplied from the Rhine River can be counted upon as certain. Detailed description of the canal water supply is contained in References 1, 2, and 3.

(3) There are no large reservoirs for water storage in the Ems Basin. Approximately 30 million m³ of water are contained within the channels of the Dortmund-Ems Canal and other canals of the basin, and could be utilized for flooding purposes at the expense of reduction of navigation and of the inherent water obstacle value of the canals themselves. Within the tidewater sections of the basin, an inexhaustible source of supply is available from the tidal reaches of the Ems River, subject to limitations of tidal variations.

c. Means of Creating Still-water Barriers and Drainage Obstacles.

(1) The water obstacle afforded by the existing waterways of the Ems River basin could be increased by utilization of one or more of the following means:

(a) Creation of still-water barriers by raising crests of existing dams or by construction of temporary dams at suitable sites, combined with confining of the flooding within desired limits by closing of culverts and other outlets in levees and road fills.

(b) Inundation of lowlands by breaching dikes and levees to create still-water barriers or to reduce cross country trafficability.

(c) Inundation of lowlands and formation of drainage obstacles by closing of drainage outlets or removal of pumping facilities.

(d) Inundation of reclaimed tidal lowlands by operation of tide gates to admit water at high tide or to prevent its drainage during low tide.

(2) Analysis was made of the flooding resulting from temporary damming to 1 m and to 3 m above mean water (MW) stages, designated respectively as "low barriers" and "high barriers." The effects of damming to other elevations can be evaluated by comparison

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with the effects of the barrier heights studied. For purpose of this study, it was assumed that the temporary damming operations were conducted during mean water conditions and that backwater pools above the structure would have level water surfaces. During high water conditions, greater flooding would be expected due to the increased water surface slope upstream from the temporary dams.

d. Effects of Still-water Barriers in Non-Tidal Reaches.

(1) General. The effects of artificial flooding created by temporary damming operations along the non-tidal reaches of the Ems River are summarized in Table 6, and the extent of the resulting inundation is indicated on the strip maps presented as Plate 15. Serial numbers of sites correspond to the bridge serial numbers of Exhibit A. It may be observed that the flooding induced by the low barriers is minor and is mainly confined to increasing depth in the channel and to flooding of old meander loops of the river. The high barriers create larger water obstacles from 0.5 to 1.5 km wide. However, in neither case is it practical to form a continuous still-water barrier except during natural flood periods. Review of the possibilities of artificial flooding in various reaches of the river follows.

(2) Source to Greven (km 295). Adequate maps were not available to permit detailed analyses of the extreme upper reaches of the river above Marienfeld (km 370). The steep river gradients prevailing in the upper reaches of the river (see profile on Plate 4), together with the height of banks (1-3 m), intensify the difficulty of creating effective still-water barriers. Blocking of bridge openings at the 7 sites indicated on Plate 15, would create short isolated shallow still-water barriers of an average width of about 500 m and length of about 1 km in the case of low barriers, and a width of about 700 m, and length of 2 to 4 km for the high barriers (see Table 6). In this section of the river, breaching of the summer levees and destruction of drainage facilities during periods of above normal river stages would create boggy conditions in the low-lying pastures and hayfields immediately adjacent to the river.

(3) Greven to Rhine (km 251). The bank heights in this reach vary from 2.5 to 4.5 m, necessitating high barriers in order to create appreciable flooding. Damming to lower elevations would create some inundation along the numerous old river meanders. Three suitable sites (Serial Nos. 59, 60, and 63) shown on Plate 15 could be used, in the region of gentler gradient upstream from Rhine, to effect flooding to average depths of about 1 m, widths of 250-400 m, and lengths of 1-10 km.

(4) Rhine to Moppen (km 176). The river flows through a narrow gorge 5-10 m deep, in the section between Rhine and Bentlage

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Weir (km 246). Bank elevations from 4.5-7.5 m above mean water stage prevail as far downstream as Salzbergen (km 240). Artificial flooding in those reaches would not be practicable. From Salzbergen to Listrup (km 226) banks up to 10 m high are found, and any artificial flooding would be confined to the beds of old meanders. Downstream of Listrup, the flood plain lies 2-3 m above MW. Some shallow artificial inundation to width of 1-2 km, mostly confined to the land adjacent to the old meanders, could be effected by creation of high barriers along that reach. Locations of suitable sites are shown on Plate 15 and information on extent of inundation is contained in Table 6.

(5) Meppen to Herbrum (km 107). The river flows through a 1.5-3 km wide valley lying approximately 2 m above the river bed, subject to flooding by winter floods but not normally covered by summer floods. In the lower part of this reach, erection of high barriers would create water obstacles with average depths of 0.5 to 0.75 m, widths of 2-4 km, and lengths of 4-17 km as shown in Table 6 and on Plate 15. In addition, as described in Exhibit B, inundation of adjacent low-lying depressions, east of the canalized river can be effected by diversion of water from the upper pools of the locks and dams at Huental (Voerssen) (km 168), Duethe (km 142), and Herbrum (km 107), through the irrigation outlets provided at those locations.

(6) Water Requirements for Still-Water Barriers. The total water required to effect the artificial flooding described in previous paragraphs and indicated in Table 6 and Plate 15 would be approximately 5 million m^3 for the low barriers or 80 million m^3 for the high barriers. At the average natural flow of 30 m^3/s expected during mean water conditions, complete filling of all the indicated flooded areas would take about 2 days for the low barriers or 31 days for the high barriers. Following are tabulated water requirements and estimated filling times for various reaches:

Reach	Water Requirement (million m^3)		Filling Time at 30 m^3/s (days-hrs.)	
	Low Barrier	High Barrier	Low Barrier	High Barrier
Source-Greven	2.4	15.6	0-22	6-0
Greven-Rheins	0.1	7.1	0-1	2-17
Rheine-Meppen	1.3	17.5	0-12	6-17
Meppen-Herbrum	1.2	40.5	0-11	15-6

Utilization of the 53 m^3/s supplied from outside sources to the canals of the basin and of the 30 million m^3 water stored in the canal channels (as described in paragraph 4-02b) would reduce the time required for filling, contingent upon the relative locations and elevations of the canal water and the still-water barriers.

(7) Combination of Still-water Barriers with Natural Water Obstacles. From Lingen (km 208) to Papenburg (km 94), the Ems

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River is flanked on the east by the Dortmund-Ems Canal and the partly completed Ems-Saaten Canal, and on the west by the Sued-Nord Canal, with many off-shooting drainage ditches interconnected by other canals with the Dutch waterways (see Plates 1 and 2). On both sides of the Ems River, there are many extensive areas of marshes and moors. Together with the river, these present a considerable cumulative obstacle, the effectiveness of which might be increased by proper combination with still-water barriers at suitable sites as indicated in preceding paragraphs.

g. Artificial Flooding Potentialities in Tidal Reaches.

In the tidal reaches of the Ems River below Herbrum (km 107), it is possible to inundate the extensive reclaimed tidal marsh regions to various depths by breaching of levees and dikes, operation of tide gates, and closing of normal drainage outlets and facilities. The possible extent of such inundation is indicated on the General Map, Plate 1. The entire area is intersected by drainage ditches and dikes and lies generally below the elevation of high tide. During low tide, much of the area would be uncovered, but due to poor drainage would probably be water soaked. Detailed analysis was not made of the flooding possibilities in this region due to lack of adequate data as to elevations of the land and the elevation and location of the dike and drainage system. Reference is made to Exhibit B and to References 1, 3 and 4 for additional information.

f. Artificial Flooding Potentialities of the Dortmund-Ems Canal.

(1) The stage in the Dortmund-Ems canal could not be raised appreciably, the maximum limit being approximately the level of the top of the lock gates. Certain possibilities exist for inundation of adjacent low-lying areas in the immediate vicinity of the canal by breaching of the canal banks at crossings of streams and in reaches where the level of the canal water surface is higher than the surrounding ground. Possible locations for such operations exist along the portions of the canal indicated as being within embankments on the Location Plan, Plate 2. The overland inundation resulting from such diversion of water would be of relatively short duration, limited by the storage available within the canal and by the rate of runoff of the water from the land into the river through natural drainage channels. Quantitative evaluation of the possible effect has not been attempted in this report due to limitations of essential basic data.

(2) In the event that the navigation use and the water obstacle afforded by the Dortmund-Ems and Ems-Weser canal become non-critical, it might be desirable to utilize the water stored in the canals to create artificial flooding along other streams of the basin and to forestall detrimental utilization by the enemy. Breaching of the canal aqueduct at the Ems River and the Glane River crossings (sites of RAF bombing during the war) would empty approximately 13 million m³

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into those rivers, provided the canal safety gates and locks along the canal were opened (See plates 1, 2, 7 and 10). The rate of outflow, however, would be too slow to produce an effective flood wave. Approximately 5 million m^3 could be discharged at the Ems River crossing and 3 million m^3 at the Glane River crossing in a 48 hour period, and several weeks would be required to completely drain the canal. The estimated rates of discharge resulting from breaching of the canal aqueducts at the Ems River and Glane River crossings follow:

Hours after Breaching	Rate of Discharge (m^3/s)	
	<u>Ems River</u>	<u>Glane River</u>
1	100	100
3	65	65
6	50	45
12	35	20
24	25	5
48	15	1

g. Artificial Flooding Potentialities of the Canals west of the Ems River. The canal system west of the Ems River, including the Sued-Nord and Ems-Vechte Canals and the Lee and Vechte Rivers afford certain potentialities for artificial flooding. Locations of those canals are shown on Plate 1, profiles are presented on Plates 8 and 9, and descriptions are contained in paragraph 2-12 and in Exhibit B. Combination of flow diversion between the interconnected waterways of the system, coupled with breaching of canal embankments and opening of outlets would inundate an extensive area along the Vechte and Lee Rivers northwest of Nordhorn as indicated on the General Map, Plate 1 and discussed in Exhibit B.

4-03 MAJOR FLOOD WAVES.

a. No large storage dams exist in the Ems River basin, thus eliminating the possibility of creating major flood waves by breaching such dams.

b. Flood waves 1 to 2 m in height and of about 12 hours duration could be created by sudden destruction of navigation locks and dams on the Ems River and Dortmund-Ems Canal. Reference is made to Exhibit B for detailed description of the effect at specific locations. Locations of dams are shown on Plates 1, 2, 4, 5, 6, and 7. Successive progressive destruction of dams could intensify and prolong the effective waves.

4-04 STREAM FLOW VARIATION.

The depth and velocity of flow in the Ems River could be increased or decreased by manipulation of the controlled outlets of the navigation dams, locations of which are shown on Plates 1, 2, 4, 5,

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6, and 7. The limited storage capacity and water supply, discussed in paragraph 4-02b would hinder the replenishment of released water and thus limit utilization of cyclic flow variation. Detailed analysis of the effect of stream flow variation was not made in this study due to the lack of essential data. Reference is made to Exhibit B for description of the effects of operation of the locks and dams upon flows in the waterways of the basin.

4-05 SUMMARY.

The artificial flooding potentialities of the Ems River Basin discussed in preceding paragraphs 4-01 to 4-04 are here summarized.

a. By means of temporary damming operations at suitable existing damsites and bridge openings, as shown on Plate 15 and in Table 6, it would be possible to create inundation extending upstream from those barriers for 1 to 17 km and 0.5-4 km wide. The average depth of overbank flooding would be from 0.5 to 1 m. Except during high water periods, the resulting overbank flooding would not present a continuous water obstacle. Temporary dams with heights of less than 3 m above mean water would not cause appreciable overbank flooding. Detailed discussion appears in paragraph 4-02d.

b. Breaching of levees and manipulation of drainage outlets would permit inundation of the low-lying reclaimed areas in the tidal reaches of the river, as shown on Plate 1 and discussed in paragraph 4-02e.

c. Shallow inundation of isolated low-lying areas could be accomplished by diversion of water from the navigation canals within the basin (See Plate 2 and paragraph 4-02f).

d. Total destruction of the navigation dams on the waterways of the basin would create flood waves 1 to 2 m high and of approximately 12 hours duration (See paragraph 4-03 and Exhibit B).

e. Manipulation of the control outlets of the navigation structures along the Ems River would produce slight variation in depth and velocity of flow, as discussed in paragraph 4-04 and Exhibit B.

f. The inadequate water supply of the Ems basin during periods of drought would limit the artificial flooding possibilities. Detailed discussion is contained in paragraph 4-02d(6).

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SECTION V

EFFECT ON MILITARY OPERATIONS

5-01 GENERAL.

The purpose of this section is to assist military planning personnel in estimating the relative value and effect of artificial floods on associated military operations such as: bridging and ferrying, trafficability, and tactical and logistical factors. The effects of artificial floods on military operations may vary greatly, depending upon the type of equipment involved, the tactical situation, and hydrologic conditions. The effects presented in this section are opinions based largely on discussions of the military effects of artificial floods given in Exhibit B, a military geography document published by the German Army.

5-02 EFFECT OF STILL-WATER BARRIERS AND DRAINAGE OBSTACLES.

a. Reference is made to paragraph 4-02 for discussion of the hydraulic features associated with formation and augmentation of water obstacles by means of temporary damming and disruption of normal drainage.

b. Bridging and ferrying operations within the backwater reaches above the temporary dams would be hindered by reason of the resulting greater width and depth of crossing. Approach trafficability would be decreased by the shallow oxbow flooding, and increased stream depths would hinder possible fording of the upper reaches of the river. The non-continuous nature of the resulting increased water obstacle (as illustrated on Plate 15) could be compensated by planning its use in combination with other natural obstacles and with tactical operations to channelize or restrict military action. The importance of the resulting reduction of velocity within the backwater reaches is minimized by the low normal natural stream velocities shown in paragraph 3-06b.

c. Continuous military support of the temporary dam installations would be necessary to prevent their destruction by enemy ground or air action. Destruction of a temporary dam would release a flood wave that would hinder crossing operations downstream from the structure for a short period and which might cause progressive failure of other downstream dams.

d. Maneuverability and trafficability over an extensive area as shown on Plate 1, would be greatly hindered by inundation of the reclaimed marshland adjacent to the tidal reaches of the Ems River below Herbrum (km 107) by admission of salt water through the dikes and by disruption of the normal drainage.

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c. Maximum effectiveness of still-water barriers and drainage obstacles would be attained by combination of artificial flooding with other natural obstacles such as the numerous marshy areas and canals. The careful selection of the most advantageous flooding locations is imperative in order to get the maximum benefit from the limited supply of water normally available for flooding purposes (See Plates 1, 3 and 15).

5-03 EFFECT OF MAJOR FLOOD WAVES.

a. Reference is made to paragraph 4-03 and Exhibit B for discussion of the hydraulic features associated with creation of major flood waves by means of destruction of existing locks and dams along the Ems River and Dortmund-Ems Canal.

b. Destruction of the navigation structures would create flood waves of up to 12 hours duration, and heights of 1 to 2 m, that would disrupt navigation, eliminate power supply of mills and industries, possibly damage or destroy downstream dams, create some local inundation, interfere with stream crossing operations and endanger floating equipment and floating bridges in the sections of the stream downstream from the destroyed installation. Reference is made to Exhibit B for detailed description of the resulting damages that might be incurred by destruction of specific dams, and to Plates 1, 2, 4, 5, 6 and 7 for locations of structures.

5-04 EFFECT OF STREAM FLOW VARIATION.

a. Reference is made to paragraph 4-04 and Exhibit B for discussion of the hydraulic effects associated with stream flow variation by means of manipulation of the discharge control outlets of the navigation dams on the Ems River and Dortmund-Ems Canal, locations of which appear on Plates 1, 2, 4, 5, 6 and 7.

b. Detrimental stream flow variation would result from opening and closing of the movable gates and other controlled outlets of the navigation dams. Sudden opening would disrupt navigation by increasing stages downstream and decreasing stages upstream from the dam, disrupt power supply of mills and industries, create moderately large waves that would cause some local inundation, endanger downstream dam structures, interfere with stream crossing operations and endanger floating equipment downstream from the sudden discharge. Sudden closing would have less effect upon navigation and power, but would hinder military crossing operation by the associated lowering of downstream stages, and raising of upstream stages. Manipulation of the control gates to create artificial cyclic oscillating waves would increase the value of the stream as a military obstacle by the action upon military floating bridging and ferrying operations. The limited water storage capacity of the dams and the limited supply of water for replenishment of the depleted storage would limit the extent and frequency of cyclic variations.

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Reference is made to Exhibit B for detailed description of the effects of opening and closing of the outlet controls of specific dams.

c. Deliberate destruction of upstream navigation or mill dams or their control gates would insure against their utilization by the enemy to produce flood waves or detrimental flow variations to interfere with military operations at a later critical period.

5-05 EFFECTS RELATED TO OTHER RIVER BASINS.

Creation of artificial flooding in the Rhine River Basin could be coordinated with similar operations on other river basins to create simultaneous or progressive water obstacles. Reference is made to reports of artificial flooding possibilities on the Rhine River, Danube River, Weser River, and Aller and Leine Rivers listed as References 16 to 20, inclusive, in the Bibliography of this report.

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TABLES

1. Equivalent English-Metric Terms
2. Hydrologic Terms and Abbreviations
3. Summary of Gage Data, Kms River (Non-Tidal Reaches)
4. Summary of Gage Data, Kms River (Tidal Reaches)
5. Discharge Data, Kms River
6. Inundation Effect of Still water Barriers

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TABLE I
EQUIVALENT ENGLISH-METRIC TERMS

To reduce A to B, multiply A by F. To reduce B to A, multiply B by G.

Unit A	Factor F	Factor G	Unit B
Miles	1,60935	.62137	Kilometers
Meters	3.2808	.30480	Feet
"	39.370	.035400	Inches
Square Miles	2.590	.3861	Square Kilometers
" "	259.000	.0038610	Hectares
Hectares	2.47101	.40469	Acres
Acres	4046.9	.00024710	Square Meters
Cubic Meters	35.3145	.028317	Cubic Feet
Cubic Feet	28.317	.73314	Liters
Acres-foot	43560.	.00002957	Cubic Feet
"	1233.5	.00001071	Cubic Meters
Cubic Feet per second	1.9835	.50417	Acres-foot per 24 hours
" Meters per "	35.3145	.028317	Cubic-foot per second
Miles per hour	1.60935	.62137	Kilometers per hour
" " "	1.4667	.68182	Feet per second
Meters per second	3.2808	.30480	" " "
" " "	2.2369	.44704	Miles per hour
Feet per second	1.097	.99113	Kilometers per hour
Tons (metric)	1.102	.9072	Tons (short)
" (long)	1.016	.9842	" (metric)
" (metric)	2205.	.0004536	Pounds
" (metric)	1000.	.001	(avoirdupois) Kilograms

TABLE 2
HYDROLOGIC TERMS AND ABBREVIATIONS
(In conformance with Ocean practice)

No.- Tidal Stage	High- Tide Stage	Low- Tide Stage	Rate of Discharge (m^3/sec)	Discharge per unit area ($l/sec-lr^2$)	Definition
H _h	H _{HTW}	L _{HTW}	H _{HQ}	H _{HQ}	Highest value ever known or observed
H _h	H _{HTW}	L _{HTW}	H _{HQ}	H _{HQ}	Highest value observed during a stated period of time
H _h	H _{HTW}	L _{HTW}	H _{HQ}	H _{HQ}	The mean high value during a stated period, derived by averaging the highest values of each unit time element (1 hr). MHW 1926/35 is average of the 10 yearly peak stages
H _h	H _{HTW}	L _{HTW}	L _Q	H _{HQ}	The mean (arithmetic average) of all observations during a stated time period
H _h	H _{HTW}	L _{HTW}	H _{HQ}	H _{HQ}	The mean low value during a stated period, derived by averaging the lowest values of each unit time element (MHW 1926/35 is the average of the 10 yearly lowest stages)
L _h	L _{HTW}	H _{HTW}	L _{HQ}	H _{HQ}	Lowest value observed during a stated period of time
L _h	L _{HTW}	H _{HTW}	L _{HQ}	L _{HQ}	Lowest value ever known or observed

Table 2

TABLE 3
SUMMARY OF GAGE DATA - EMS RIVER (Non-tidal Reaches)

Main Source: Jahrbuch fuer die Gewässer Kunde des Deutschen Reichs, Abflussjahr 1935

Gage	OSOS Map Series	"Nord du Guerre" Grid	Km. Above Mouth of River	Drainage Area Sq. Km.	Gage Zero m/MS	Item*	Date or Period of Record	River Stage in Centimeters above Gage Zero														
								Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Year		
Groven	F2 3911	A9188	295.2	2898	32.72	HHW	Nov 1890													797		
						HW	1926-35														744	
						MHW	"	451	468	578	478	433	463	354	273	266	221	196	333	638		
						NW	"	304	322	395	360	293	305	207	168	158	140	139	197	248		
						MNW	"	191	214	242	250	200	203	135	111	99	100	99	119	89		
						HW	"														62	
Zweize (Lower Lock)	H2 3710	V7910	250.5	3740	24.18	HHW	Nov 1890													820		
						HW	1926-35															759
						MHW	"	387	382	476	380	337	353	282	226	231	205	192	270	551		
						NW	"	265	271	323	295	246	256	199	175	169	161	159	194	226		
						MHW	"	191	203	225	221	191	193	157	142	136	133	134	148	126		
						HW	"														106	
Vorsen (upstream of weir)	H1 3209	V6759	168.4	8404	6.69	HHW	June 1926													516		
						HW	1926-35															516
						MHW	"	294	331	387	325	293	304	267	196	191	164	159	232	428		
						NW	"	211	228	284	254	209	213	162	138	125	120	120	154	182		
						MNW	"	152	154	196	190	149	146	112	103	95	94	99	107	89		
						HW	July 1930														80	
Rupenburg	H2 2810	Q7501	93.8		-5.00	HHW	Jan 1901													846		
						HW	1926-35														819	
						MHW	"														782	
						NW	"	643	637	653	636	632	642	638	639	644	639	639	650	641		
						MHW	"														502	
						HW	"														457	
						MHW	Jan 1922														439	
						NW	Jan 1926														757	
						MHW	1926-35														757	
						NW	"														642	
						MHW	"	489	495	534	500	477	479	461	457	459	456	458	475	478		
						NW	"														388	
						MHW	"														350	
						NW	Oct 1908														339	

*See Table 2 for definition of symbols

Table 3

TABLE 4
SUMMARY OF GAGE DATA - RME RIVER (Tidal Reaches)

Source: Jahrbuch fuer die Gewasser Kunde des Deutschen Reichs, Abflussjahr 1938

Gage	Map Series 4516 4515 32 2710	"Nord du Guerre" Grid Q7913	Ea. Above Mouth of Rme River 77.0	Drainage Area Sq. km. -	Gage Zero m/NE -5.00	Item	Date or Period of Record	River Stage in Centimeters above Gage Zero													
								Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	June	Jul.	Aug.	Sept.	Oct.	Year	
Leersort						RTW	Mar 1906													1004	
						RTW	1931-35														884
						RTW	.														516
						RTW	.	641	630	644	646	626	639	637	640	650	643	646	656	642	
						RTW	.														498
						RTW	.														561
						RTW	Nov 1916														396
						RTW	Oct 1926														676
						RTW	1931-35														646
						RTW	.														590
						RTW	.	407	399	417	415	389	398	388	386	396	391	398	412	400	
						RTW	.														317
						RTW	.														290
						RTW	Nov 1916														210
Boden (New Lock)	11 2809	Q6327	50.8		-5.00	RTW	Mar 1926													1018	
						RTW	1926-35														933
						RTW	.														877
						RTW	.	629	621	628	618	615	624	624	630	636	632	631	641	628	
						RTW	.														479
						RTW	.														463
						RTW	Jan 1905														161
						RTW	Dec 1894														769
						RTW	1926-35														669
						RTW	.														577
						RTW	.	335	329	330	319	313	317	313	322	328	326	336	350	317	
						RTW	.														293
						RTW	.														169
						RTW	Jan 1937														141

See Table 3 for definition of symbols

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TABLE 5
DISCHARGE DATA - EMS RIVER
Period 1926-35

Gage	Oreven	Rheims	Perren
Kr., above Mouth Drainage Area: (Km ²)	295 2695	250 3740	168 8404
Discharge (m ³ /s) Discharge/unit area (l/s/km ²)			
$\frac{HHQ}{HHq}$	$\frac{580^a}{200}$	$\frac{460^{(2)}}{123}$	$\frac{370^a}{44.1}$
	26 Nov. 1890	27 Nov. 1890	6 Jan 1926
$\frac{HHS}{HHS}$	$\frac{320^a}{110.5}$	$\frac{358^{(2)}}{95.7}$	$\frac{370^a}{44.1}$
$\frac{MHQ}{MHq}$	$\frac{116^{(1)}}{40.0}$	$\frac{195^{(2)}}{52.1} \quad \frac{188^{(1)}}{50.3}$	$\frac{230^{(1)}}{27.4}$
$\frac{MHS}{MHS}$	$\frac{18.5^{(1)}}{6.38}$	$\frac{32.3^{(2)}}{8.62} \quad \frac{27.8^{(1)}}{7.23}$	$\frac{68.0^{(1)}}{8.10}$
$\frac{LHQ}{LHq}$	$\frac{2.40^{(1)}}{0.83}$	$\frac{3.07^{(2)}}{1.36} \quad \frac{4.20^{(1)}}{1.12}$	—
$\frac{LSH}{LSH}$	—	$\frac{3.14^{(2)}}{0.82}$	—
$\frac{LHQ}{LHq}$	—	—	—

(1) Flow at MHS, MH, MHS from Jahrbuch fuer die Gewässerkunde des Deutschen Reichs - 1938.

(2) Mean and extreme discharges (Q) tabulation in 1938 Jahrbuch.

* Flow at mean and extreme stages of tables 3 and 4 obtained by application of discharge rating curves from Plate 14.

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SECURITY INFORMATION

TABLE 6
LMS RIVER
FLOODATION EFFECT OF STILLWATER BARRIERS

Serial No.	Km above mouth of Rm	Map series 4015 4016	*bord de Guerre* Grid	Location and Description *	Low stillwater barrier (1m MW)						High stillwater barrier (3m MW)					
					Road level m/MS	Length km	Aver. width m	Aver. depth m	Pond area km ²	Vol. 10 ⁶ m ³	Road level m/MS	Length km	Aver. width m	Aver. depth m	Pond area km ²	Vol. 10 ⁶ m ³
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17
	392 to 372			Maps of area not available												
79AC	358.3	P2 4015	374701	S. of Marlesfeld Rd. Br. 2 spans Total opening 24 m Stream width 18 m	65.0	2	500	0.5	1	0.5	66.6	4.5	900	1	4	4.0
	Must hold along 600 m of road embankment. May require blocking of ditch on left bank.															
792	365.5	P2 4015	350710	S.W. of Marlesfeld Rd. Br. 2 spans Total opening 28 m Stream width 18 m	63.0	1.5	500	0.4	0.75	0.3	65.0	5	600	1.1	3	3.3
	Must hold along 800 m of road embankment. May require blocking of drainage ditches under left embankment.															
797	363.3	P2 4015	334720	S. of Harszwinkle Rd. Br. 1 span Total opening 15.3 m Stream width 12 m	61.6	1	400	0.4	0.4	0.2	63.6	3	500	1.2	1.5	1.8
797	359.4	P2 4015	304735	S.W. of Harszwinkle Rd. Br. 1 span Total opening 22 m Stream width 12.2 m	59.5	3	300	0.5	2.7	1.4	61.5	4.5	900	1.1	4	4.4
	Must hold along 1 km of embankment. May have to block opening under embankment on left bank of river.															
	399 to 346			No significant flooding												
79H	345.6	P2 4015	174730	Warendorf Rd. Br. 3 spans Total opening 30.5 m Stream width 19.5 m	51.4						53.4	4	500	1	2	2.0
	No significant flooding															
	346 to 327			No significant flooding												
79C	326.9	P2 4012	035763	Telgte Rd. Br. 2 spans Total opening 725 Stream width 10-20 m	45						47	1	300	0.5	0.3	0.15
	No significant flooding															
	327 to 271			No significant flooding												
	Flooding on left bank only. Requires blocking of bridge on river and bridge on mill channel															

*See Exhibit A for details

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SECURITY INFORMATION

TABLE 6
ZEM RIVER
FLOODING EFFECT OF STILLWATER BARRIERS

Serial No.	Km above mouth of Rm	Map series 0608 4416 4416	Word de Ouerre* Grid	Location and Description*	Low stillwater barrier (1m/MW)						High stillwater barrier (3m/MW)					
					Pond level m/FM	Length km	Aver. width m	Aver. depth m	Pond area km ²	Vol. 10 ⁶ m ³	Pond level m/FM	Length km	Aver. width m	Aver. depth m	Pond area km ²	Vol. 10 ⁶ m ³
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17
59	270.5	P2 3011	866957	<u>Hamelton Rd. Br.</u> 3 spans Total opening 76.8 Stream width 22.5	No significant flooding						34.8	1	250	0.5	0.25	0.12
60	266.3	M2-P2 3711	842041	<u>Mosun Rd. Br.</u> 3 spans Total opening 76.6 m Stream width 34.0 m	No significant flooding						34	5	400	1	2	2.0
59	253.4	M2-P2 3710	808080	<u>Abshire RR Br.</u> 6 spans Total opening 103 m Stream width 54 m	31.50	2	300	0.2	0.6	0.12	33.5	10	400	1.25	4	5.0
251 to 246				Rhine to Bentlage Veir - River runs in gorge 5-10 m deep. No flooding probable.												
246 to 240				Bentlage to Salsbergen - Banks are 4.5 to 7.5 m high. No flooding probable.												
52	226.0	3610	730220	<u>Lieser Weir</u>	Site not considered suitable. Flooding confined to channel or old meander beds.											
50	214.4	M1 3309	711304	<u>Elbergen RR Br.</u> 3 spans Total opening 104 m Stream width 116 m	22.5	4.5	600	0.5	2.7	1.3	24.8	8	1100	1.2	8.8	10.6
49	213.9	M1 3309	705306	<u>Haukenfecht Weir</u>	22.5	24.5 Flooding would cover same area as Elbergen Br., but would require erection of temporary levees on left bank between Weir and Ras-Vechte Canal, approximately 500 m.										
48	208.3	M1 3309	705352	<u>Lingen Rd. Br.</u> 2 spans Total opening 66 m Stream width 56 m	16	No significant flooding.					20	Flooding of old meanders. Requires blocking of 2 bridges over Dortmund-Ras Canal or construction of temporary levee approximately 200 m long between river and canal.				
45	202.0	M1 3309	682390	<u>Altenslingen Rd. Br.</u> 5 spans-Total opening 42 m Stream width 43.5 m	16.60	No significant flooding.					18.60	Flooding of old meander beds.				
43	192.3	M1 3309	669446	<u>Dalmer Rd. Br.</u> 8 spans Total opening 72.3 Stream width 63.3	14.5	No significant flooding.					16.5	Flooding of old meander beds and low ground immediately adjacent to channel.				
41	175.7	M1 3309	698355	<u>Walden Rd. Br.</u> 6 spans Total opening 64 m Stream width 60.2	11.4	No significant flooding.					13.4	6	1400	0.8	8.5	6.9
											Must hold along 1 km of RR embankment or flow will be diverted overland and re-enter the river near Vaaraken Weir.					

*See Exhibit 2 for details

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SECURITY INFORMATION

TABLE 6
RMS RIVER
FOUNDATION EFFECT OF STILLWATER BARRIERS

Serial No.	Km above mouth of Rm	Map series 4416 4414	"Nord de Guerre" Grid	Location and Description*	Low stillwater barrier (1m/MW)						High stillwater barrier (3m/MW)					
					Pond level m/KM	Length km	Aver. width m	Aver. depth m	Pond area km ²	Vol. 10 ⁶ m ³	Pond level m/KM	Length km	Aver. width m	Aver. depth m	Pond area km ²	Vol. 10 ⁶ m ³
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17
39	168.4	M1 3209	672595	<u>Veerssen Weir</u>	2 weirs at this point; 1 on meander and 1 on cut-off. Site not considered suitable for flooding.											
34	164.0	M1-M2 3209	662627	<u>Wegene Rd. Br.</u> 4 spans Total opening 110 m Stream width 54 m	5.6						10.6					Flooding of old meander beds.
33	158.6	M1-M2 3209	671662	<u>Harren Rd. Br.</u> 7 spans Total opening 144.5 Stream width 31 m	8.5						10.5	4.3	2200	0.75	9.5	7.1
32	151.0	M1 3109	681703	<u>Hilster Weir</u>	Weir bypassed by canal. Low lying plain to left of Weir channel. Site not considered suitable.											
28	143.2	M1 3109	713746	<u>Lathen Rd. Br.</u> 1 main span Total opening 91.9 Stream width 70 m	6.0						8.0	6.8	3600	0.6		
27	141.6	M1 3109	716760	<u>Deathe Weir</u>	Low ground to left of weir. Site not considered suitable.											
25	128.0	M1 3009	708812	<u>Steinbild Rd. Br.</u> 2 spans Total opening 125.4 m Stream width 55.4 m	5.0						7.0	10	2600	0.5	26	13.0
20	118.4	M1 3009	709875	<u>Heede Rd. Br.</u> 5 spans Total opening 206.8 m Stream width 56.0 m	5.0	4	600	0.4	2.4	1.0	6.9	17	2400	0.5	40.8	20.4
					Old bed on left bank.											
					5.0	3	200	0.3	0.6	0.2	Includes area covered by Steinbild High Barrier.					
					Old bed on right bank.											

*See Exhibit A for details

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PLATES

1. General Map
2. Location Plan, Dortmund-Ems Canal
3. Physiographic Diagram
4. Profile, Ems River (Lippling-Mesum)
5. Profile, Ems River (Mesum-Hoede)
6. Profile, Ems River (Hoede-Borkum Island)
7. Profile, Dortmund-Ems & Ems-Seiten Canals
8. Profile, Sued-Nord & Ems-Vechte Canals
9. Profile, Vechte River & Lee River
10. Profile, Mittelland Canal
11. Cross-Section, Dortmund-Ems Canal
12. Stage Duration Curves (Non-Tidal Reaches)
13. Stage Duration Curves (Tidal Reaches)
14. Discharge Rating Curves
15. Inundation by Still water Barriers

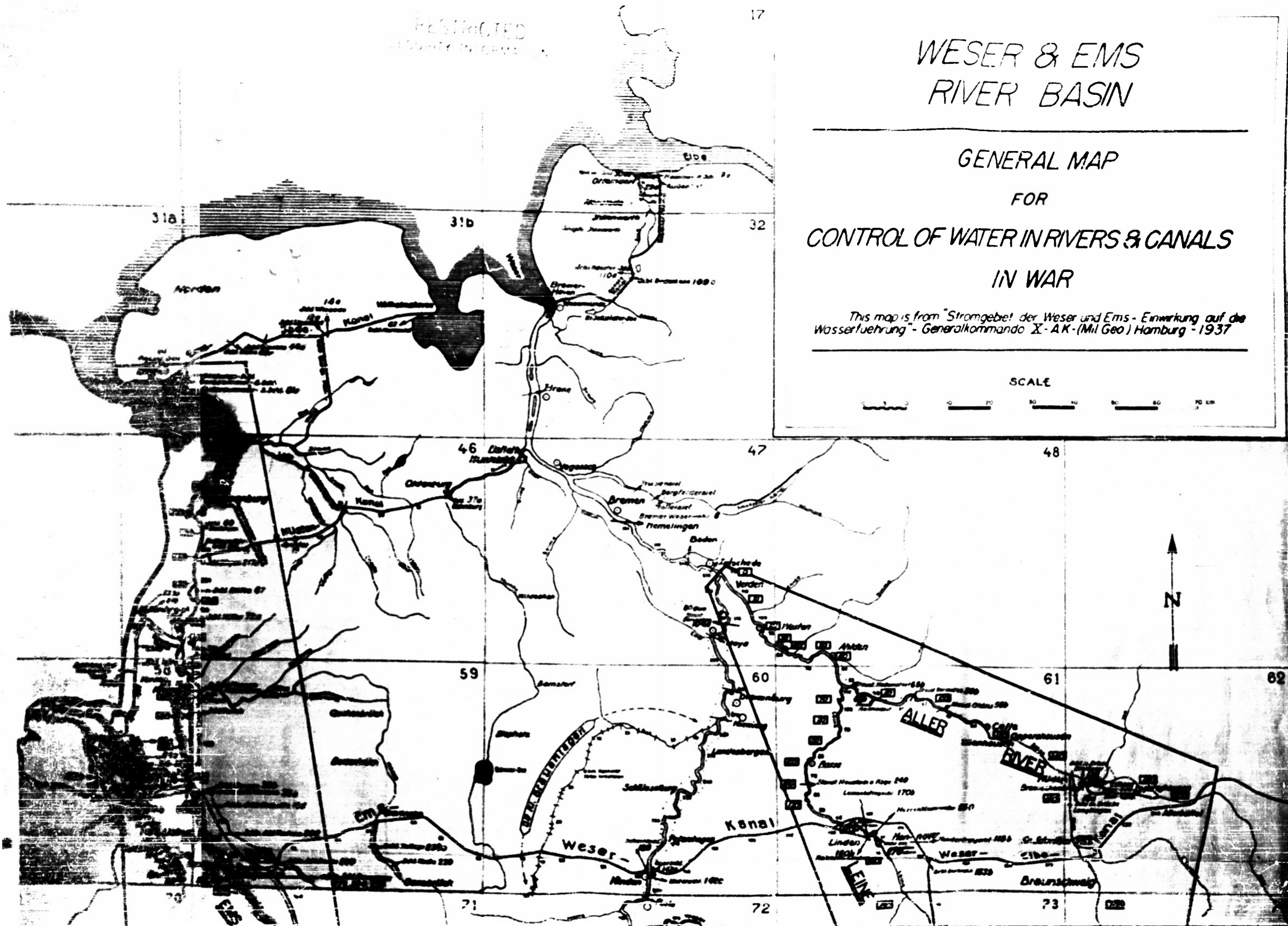
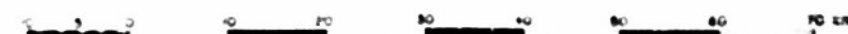
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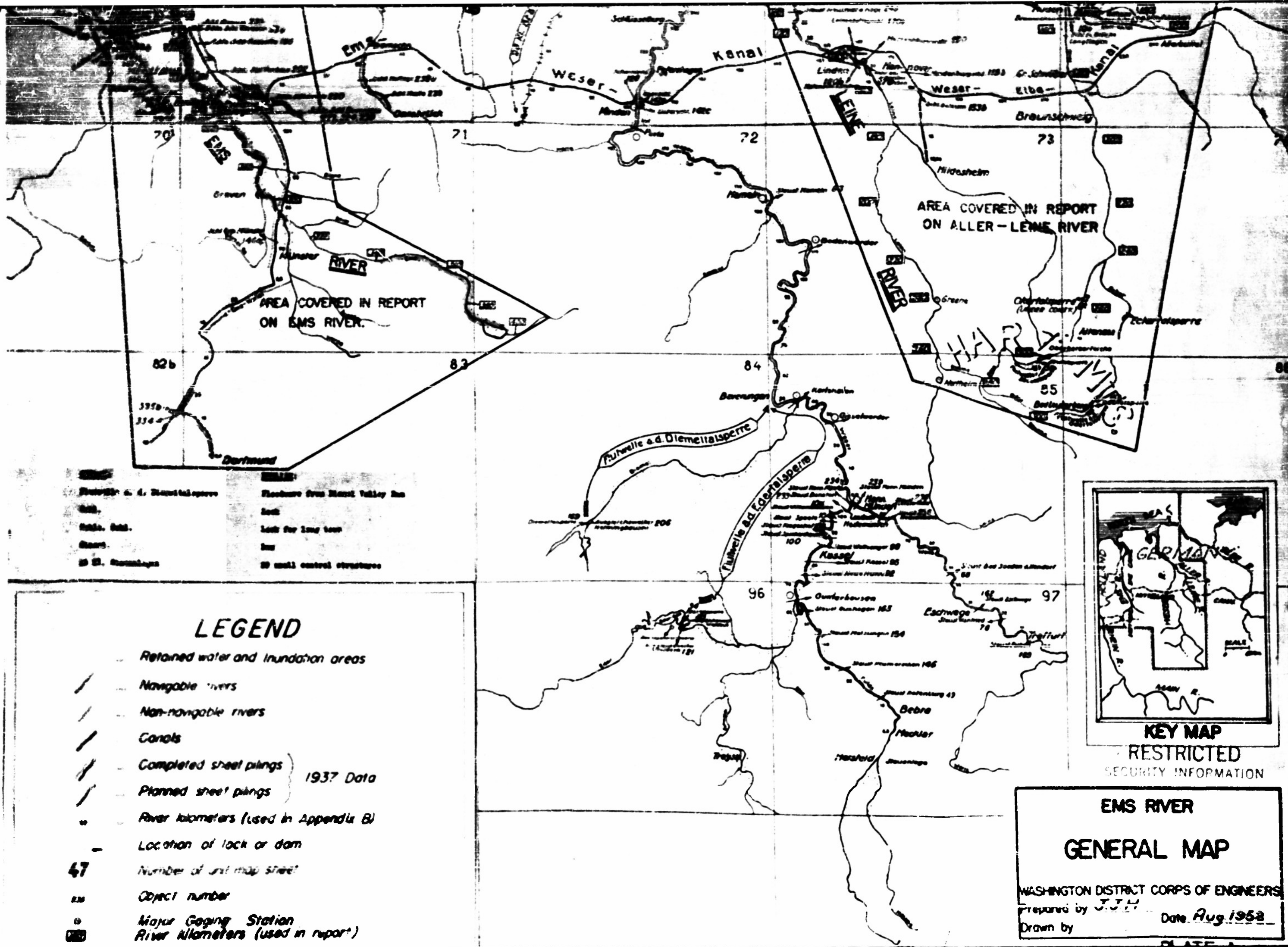
WESER & EMS RIVER BASIN

GENERAL MAP FOR CONTROL OF WATER IN RIVERS & CANALS IN WAR

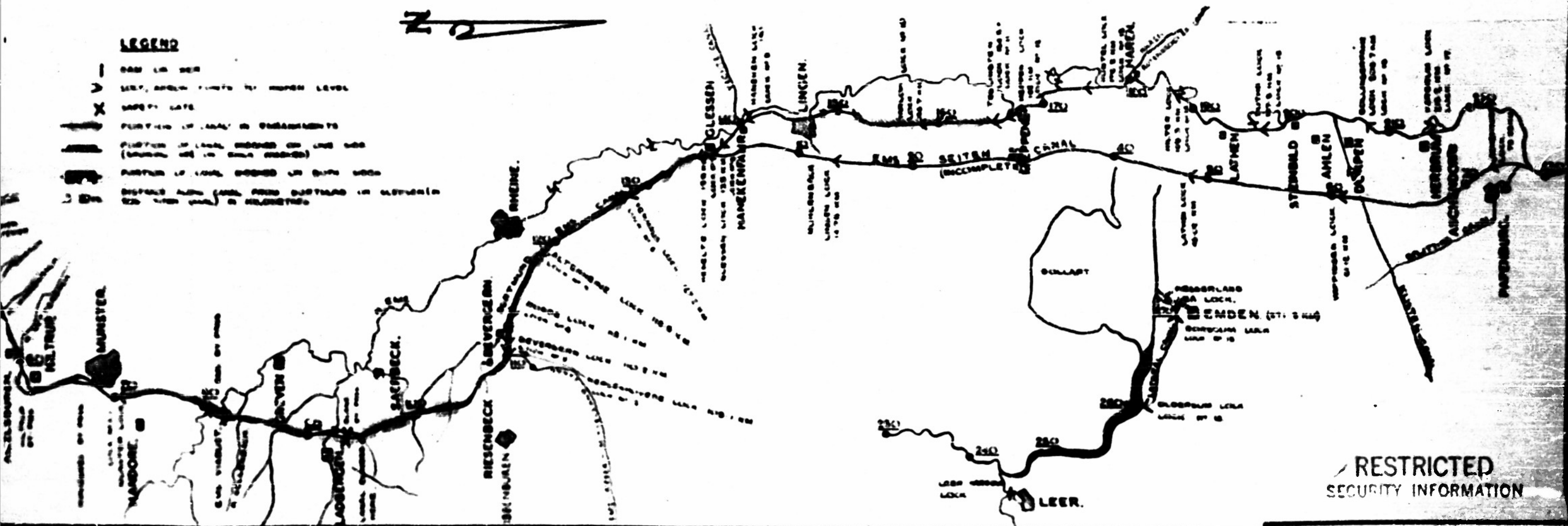
This map is from "Stromgebiet der Weser und Ems - Einwirkung auf die Wasserführung" - Generalkommando X-AK-(Mil Geo) Hamburg - 1937

SCALE





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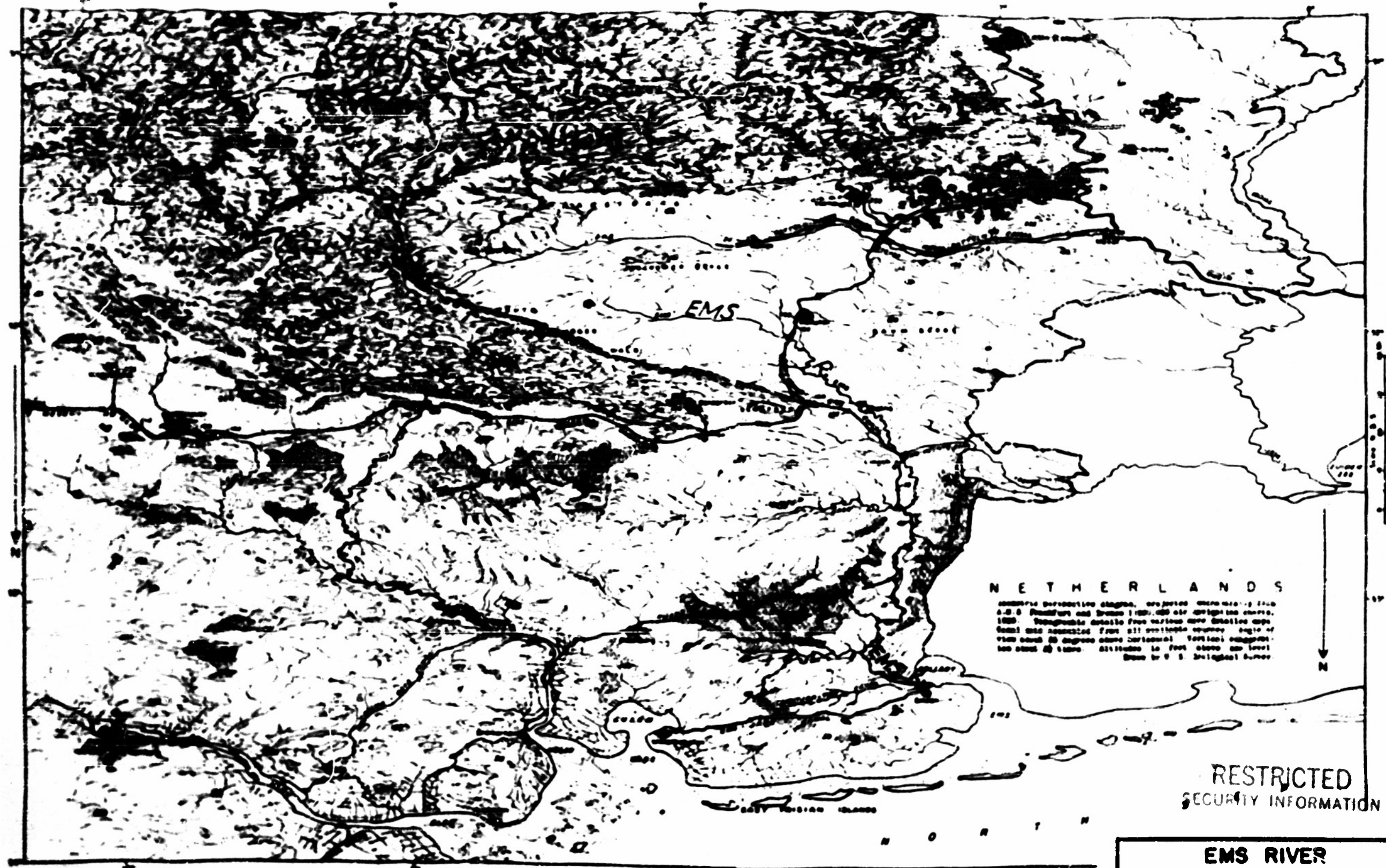


REPRODUCED FROM "REPORT ON RIVER EMB.," O.S.I. (32)
MAIN HQ., 21ST ARMY GROUP. DECEMBER, 1945.

**EMS RIVER
LOCATION PLAN
DORTMUND EMS CANAL**

WICKHAM DISTRICT CORPS OF ENGINEERS
Prepared by *L.F.H.* Date *Aug 1952*
Drawn by *-----*

PLATE 2



--- Transportation routes shown
by width of line (1/2 inch = 100 miles)

--- Elevation in feet

--- Contour lines
shown at 1000 foot intervals

--- Elevation in feet

--- Drainage
in area

--- Drainage

--- Cut in area

--- Pumping plant or water works

--- Storage dam
(top shown in black)

--- Tide line

--- Marsh, wet, or
peat bog

REPRODUCED FROM "NAVIGABLE WATERWAYS OF GERMANY"
S.E.S. 125, O.C.E., U.S.A., AUGUST, 1944.

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ELEVATION IN METERS + N.M.

400

380

360

340

320

300

KILOMETERS ABOVE MOUTH OF EIMS RIVER

Lepping Rd. Br. & Main

Riesberg Rd. Br. & Main

(185) N. Rhede Rd. Br.

(184) S. Marientfeld Rd. Br.

(183) S.W. Marientfeld Rd. Br.

(182) S. Harsenwinkel Rd. Br.

(181) S.W. Harsenwinkel Rd. Br.

(180) Grotten Rd. Br.

(179) S.W. Grotten Rd. Br.

(178) Waindorf Rd. Br.
(177) " " Rd. Br.

(176) Eilen Rd. Br.

(175) Nichte Rd. Br.
(174) " " Rd. Br.

(173) H.S. Lengen Rd. Br.

(172) Dorbaum R.R. Br.

(171) Githrup Rd. Br.

(170) Githrup Rd. Br. & Canal

(169) Sandmühlbach Vltir.
(168) Grotten Rd. Br.

(167) Grotten Rd. Br. & edge

Wiederbrück R.R. Br.
Rd. Br.
Rd. Br.
R.R. Br.
Rd. Br.
Rd. Br.
R.R. Br.
Rd. Br.
Rd. Br.
R.R. Br.

End of Navigation

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SECURITY INFORMATION

LEGEND



River junction  Canal junction 

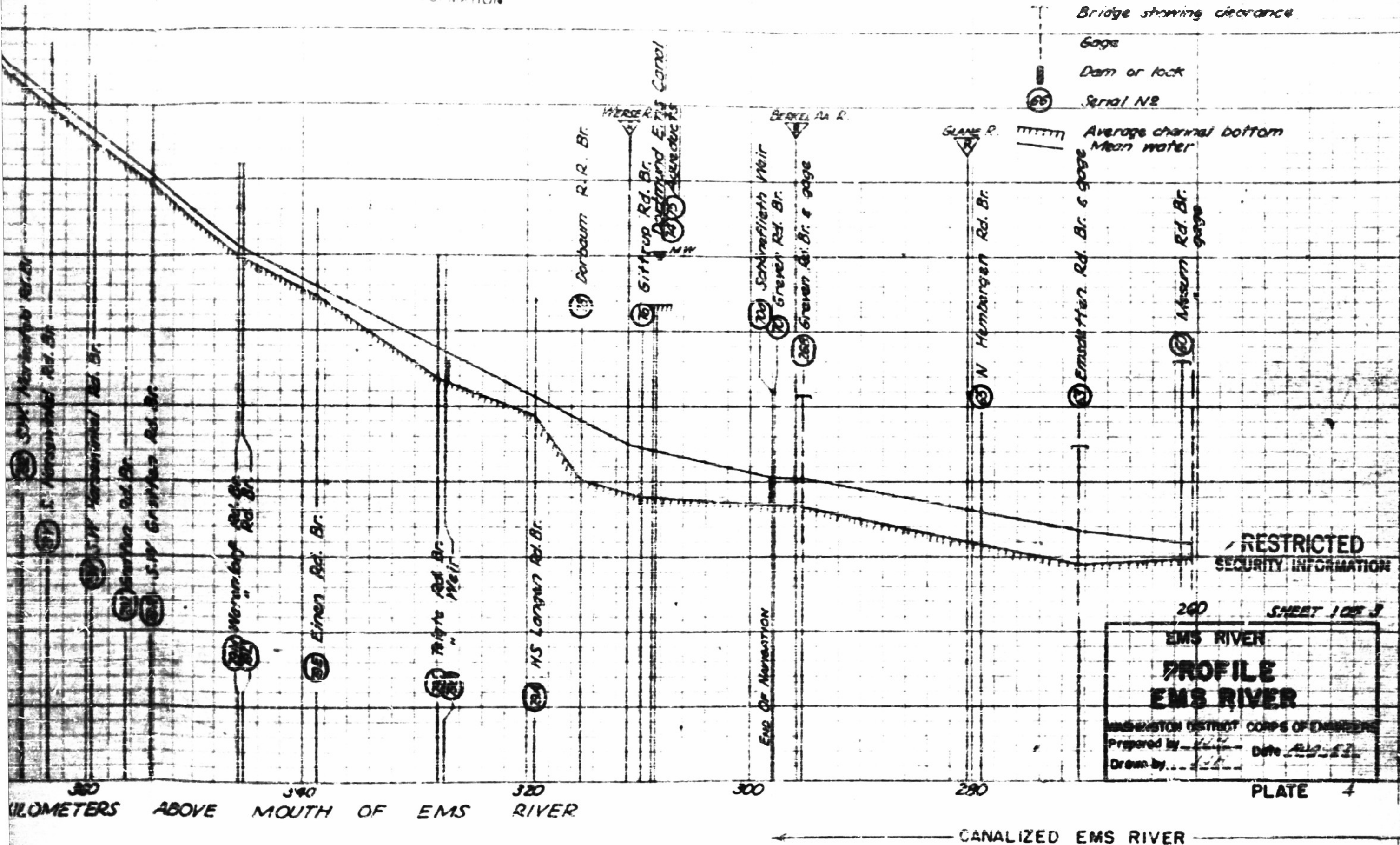
Bridge showing clearance 

Gage 

Dam or lock 

Serial No 

 Average channel bottom
 Mean water



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SECURITY INFORMATION

260 SHEET 1 OF 3

EMS RIVER
PROFILE
EMS RIVER




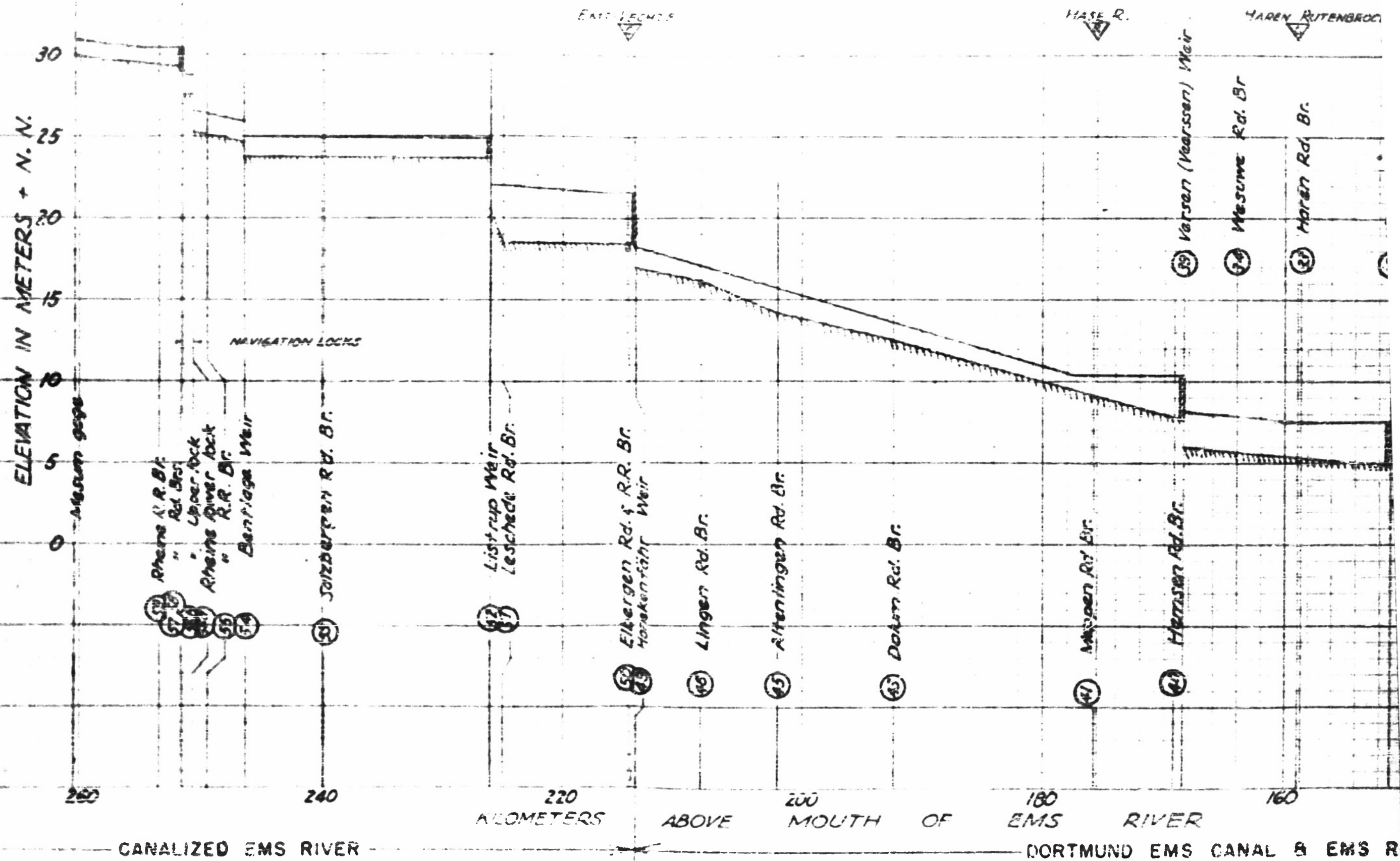
WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by  Date 
Drawn by 

PLATE 4

CANALIZED EMS RIVER

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SECURITY INFORMATION



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SECURITY INFORMATION

LEGEND

△ River junction

▽ Canal junction

Bridge showing clearance

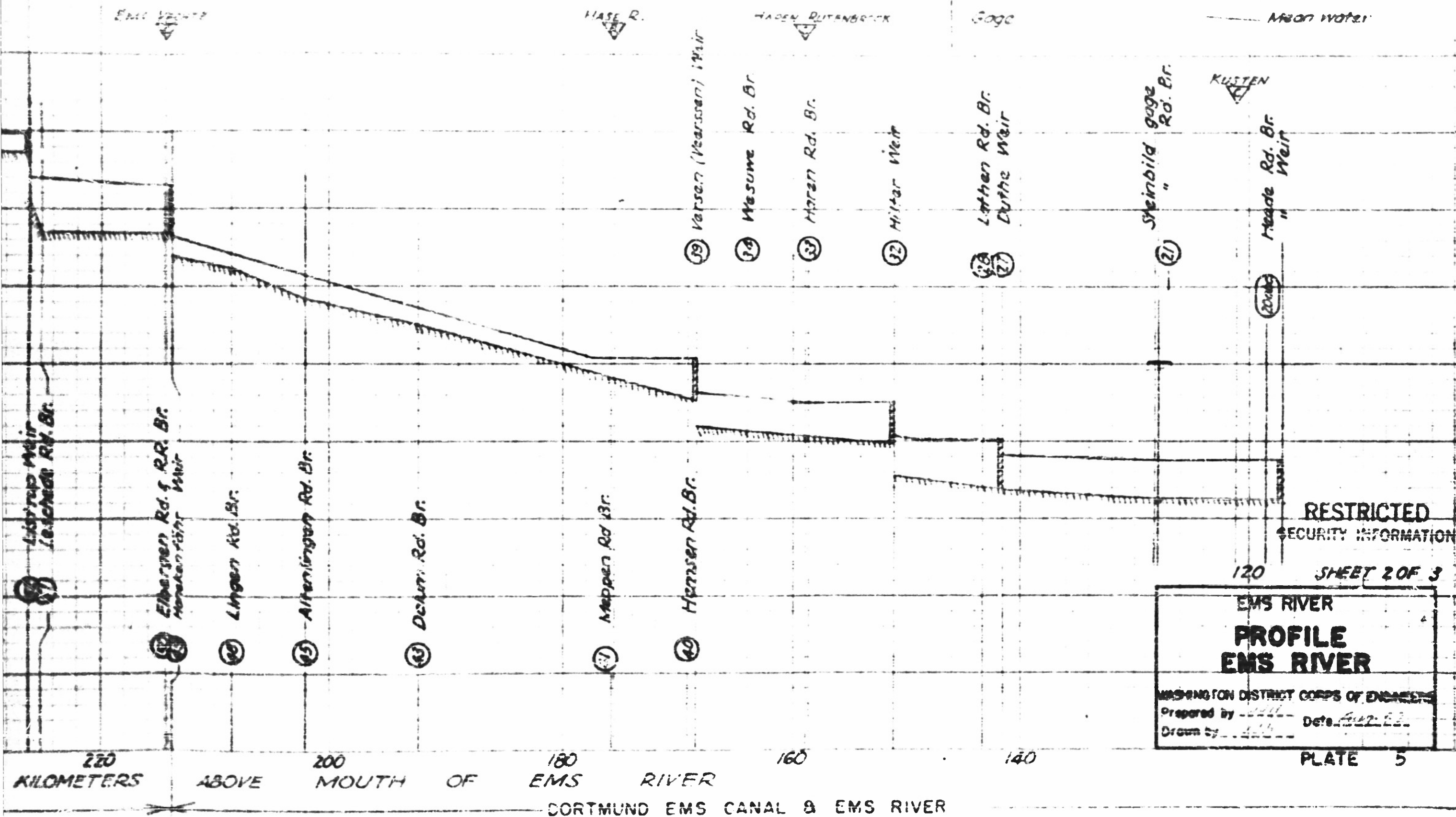
Gage

○ Dam or lock

① Serial No

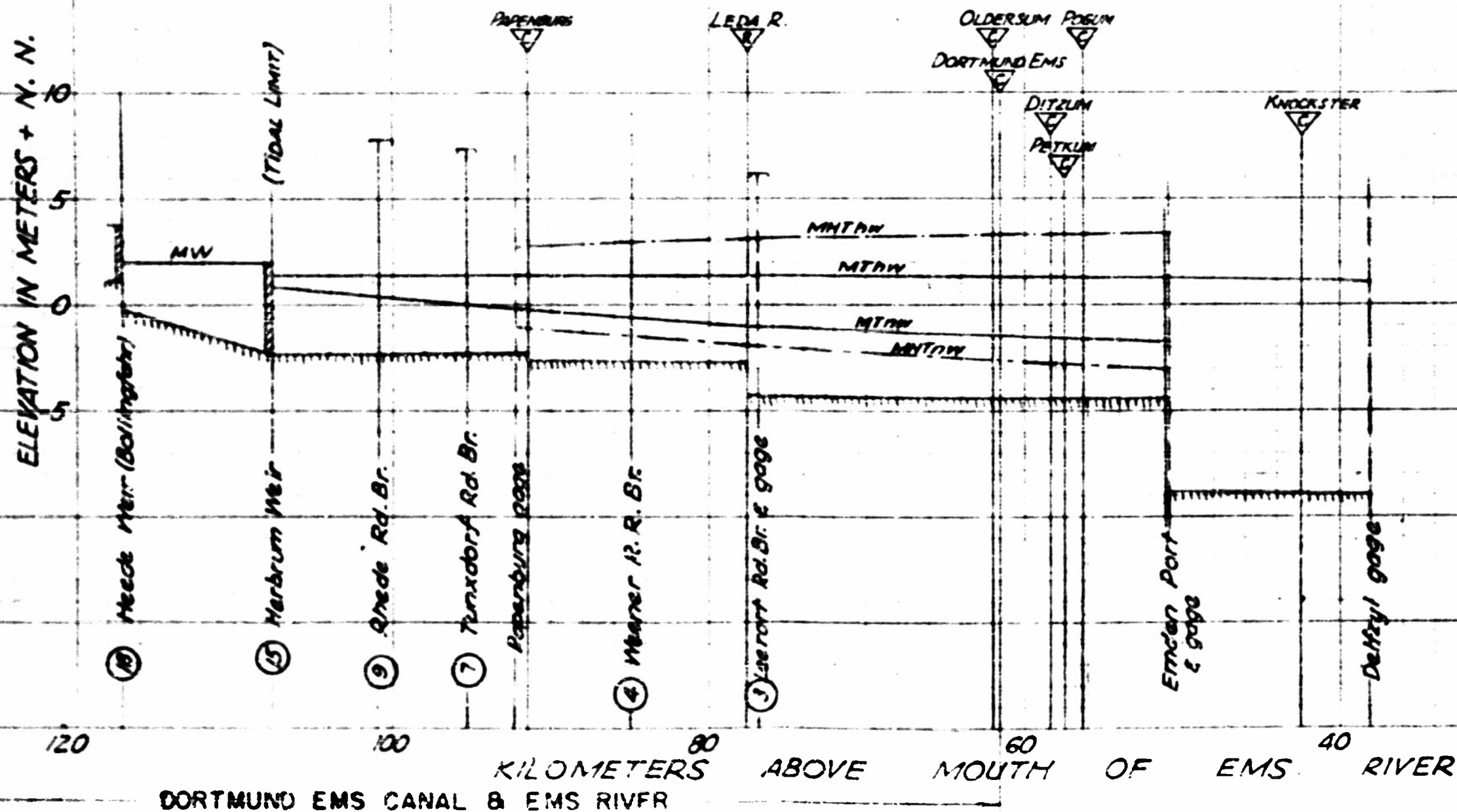
— Average channel bottom

— Mean water



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SECURITY INFORMATION

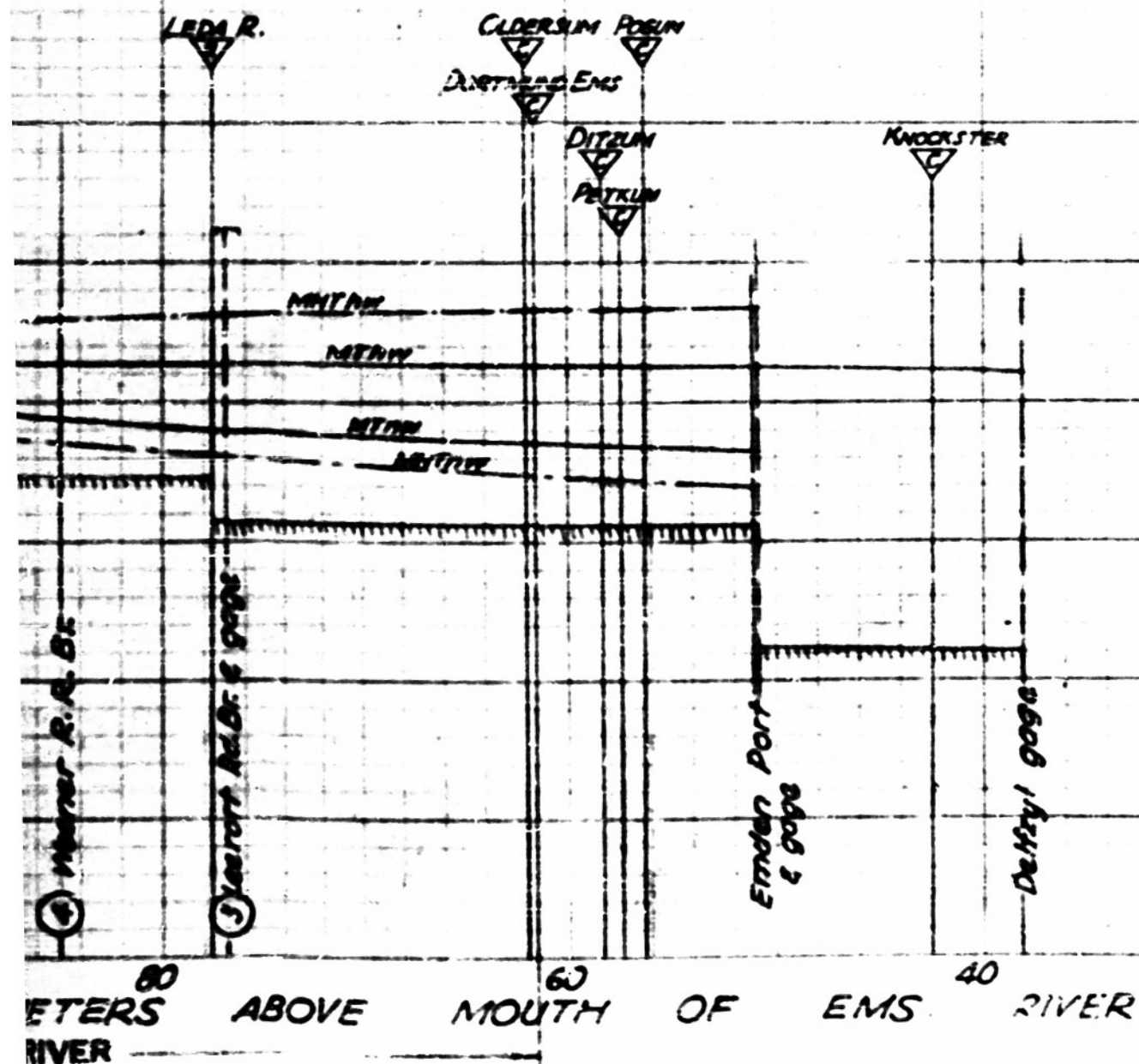
NOTE:
MHTW - Mean high tidal high water
MTHW - Mean tidal high water
MTLW - Mean tidal low water
MNTW - Mean low tidal low water



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NOTE:
MHTW - Mean high tidal high water
MTHW - Mean tidal high water
MTLW - Mean tidal low water
MLTLW - Mean low tidal low water

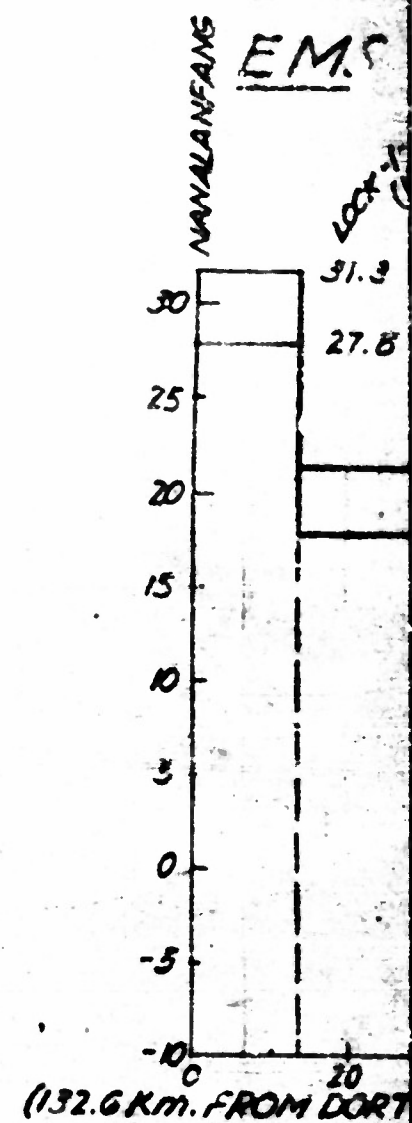
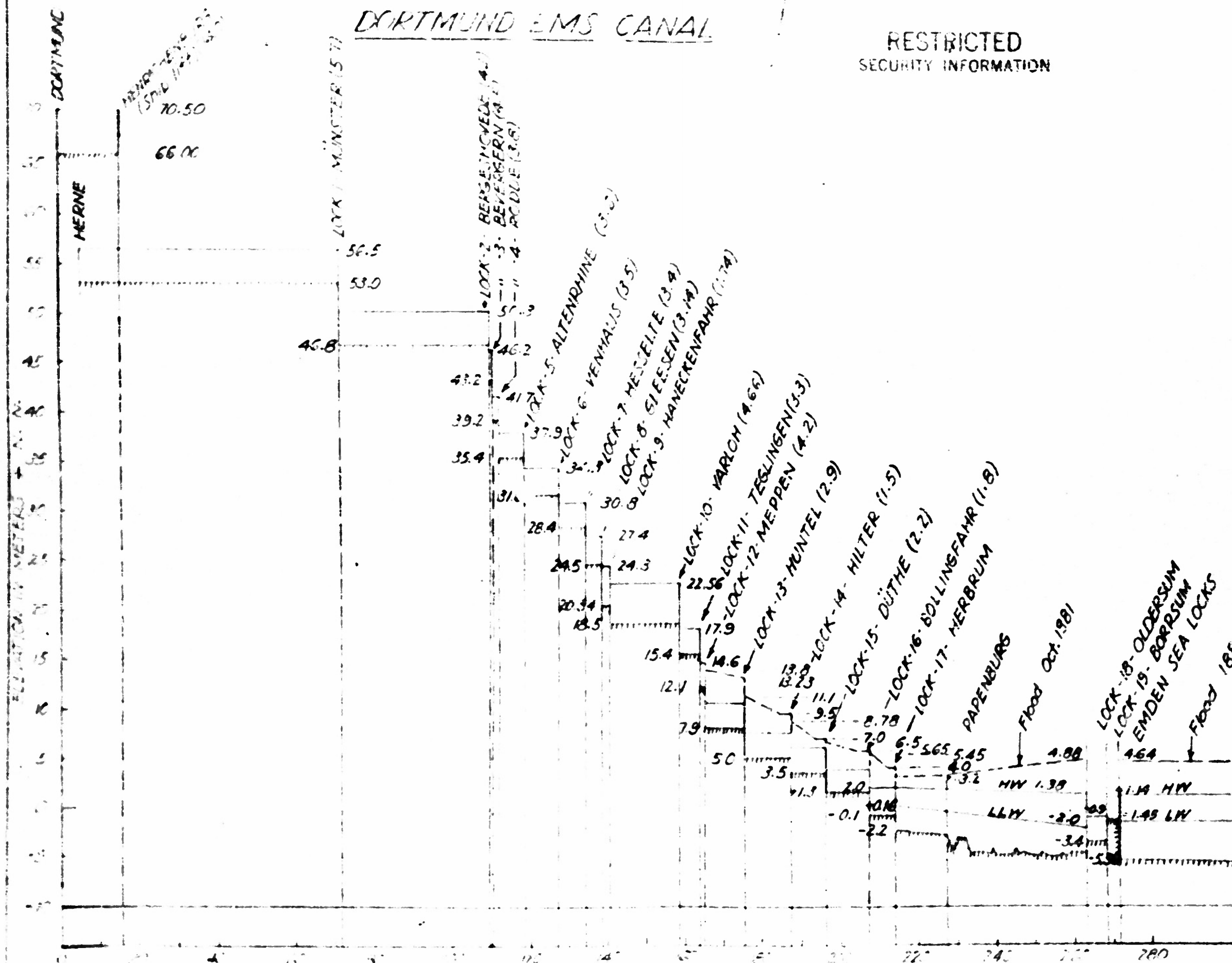
LEGEND
River junction
Canal junction
Bridge showing clearance
Gage
Dam or lock
Serial No
Average channel bottom
Mean Water
Mean Low water
Mean High water



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SHEET 3 OF 3
EMS RIVER
PROFILE
EMS RIVER
WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by J.H. Date Aug 52
Drawn by J.H.
PLATE 6

DORTMUND EMS CANAL

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SECURITY INFORMATION

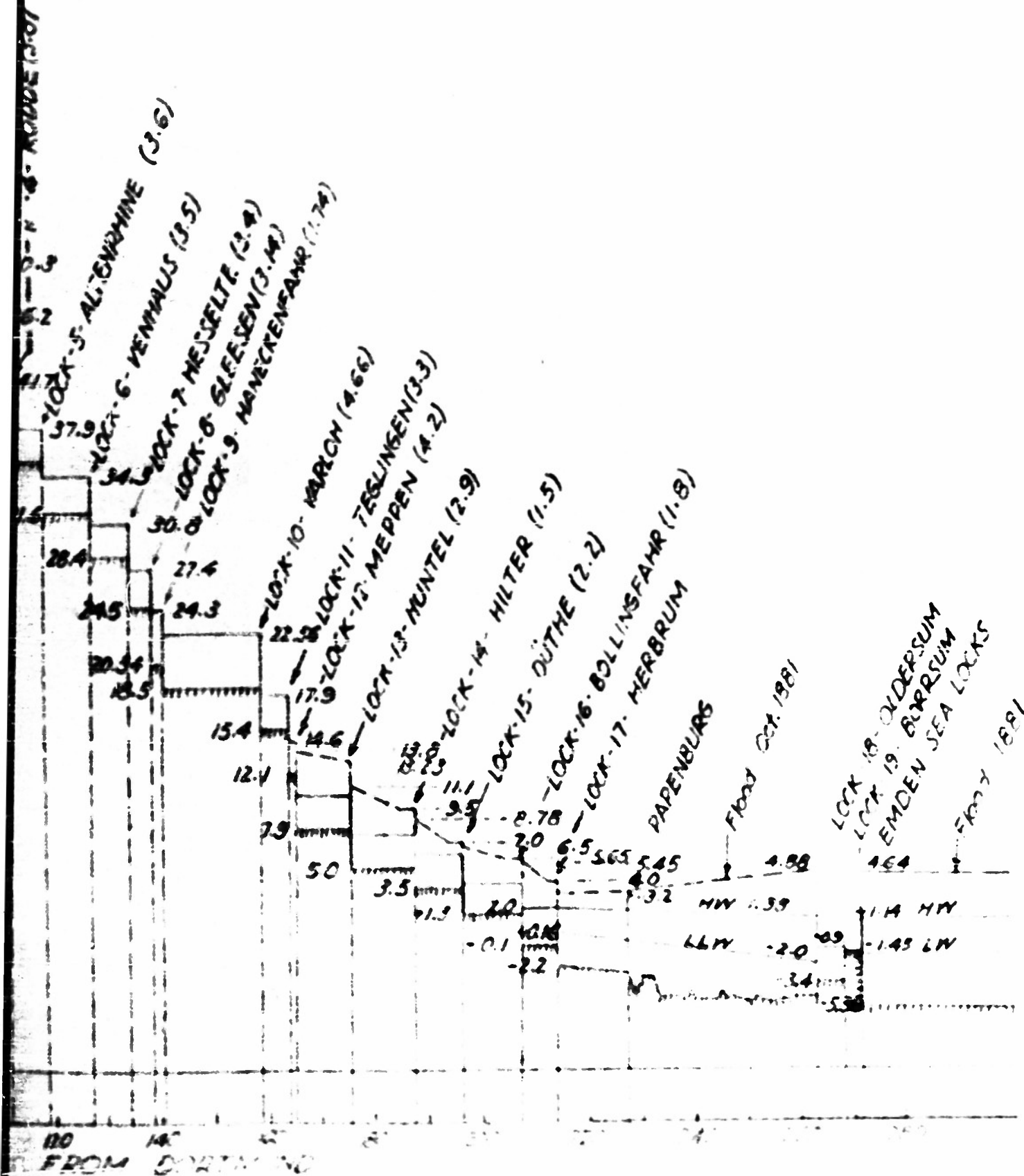


LEGEND

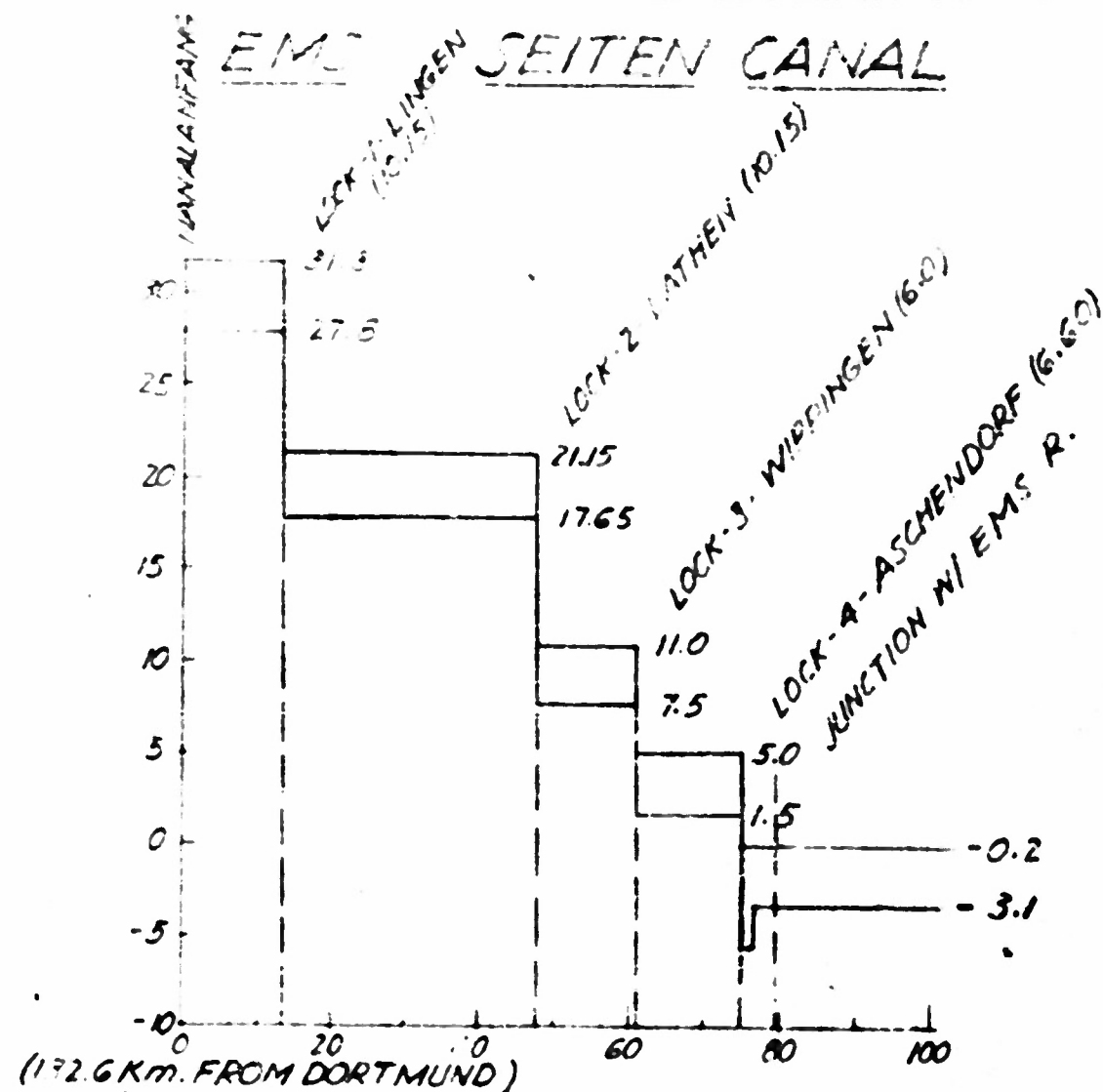
NOTE:
Berges
old pub
now be
Traco
River
HQ
1945

EMS CANAL

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EMS SEITEN CANAL



LEGEND

- Water level
- Bed level
- Flood level

NOTE: Bed levels below Bergeshore are from an old publication. Depths may now be increased.

Traced from "REPORT ON RIVER EMS," S.S.I. (RE) Main Hq, 21st Army Group. Dec. 1945.

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EMS RIVER PROFILE DORTMUND-EMS & EMS-SEITEN CANALS

WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by J.H.H. Date Aug 58
Drawn by -----

ELEVATION IN METERS +N.N.

25
20
15
10
5

60

40

20

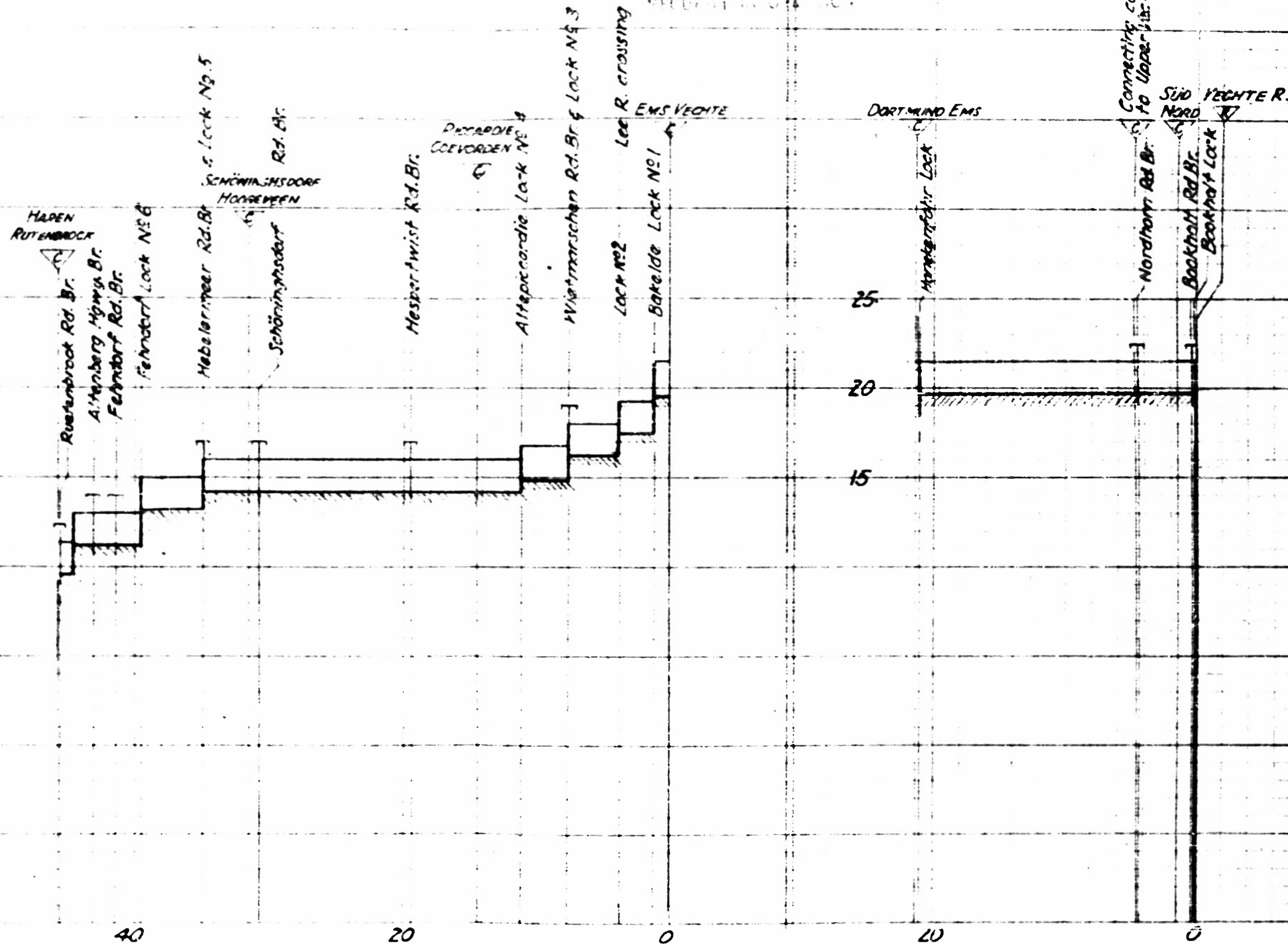
0

20







0

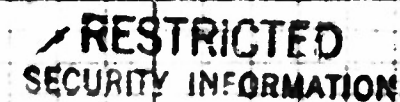
DISTANCE IN KILOMETERS
SÜD-NORD CANAL

DISTANCE IN KILOMETERS
EMS-VECHTE CANAL



LEGEND

	River junction		Canal junction
	Bridge showing clearance		
	Dam or lock		
	Average channel bottom		
	Mean water		

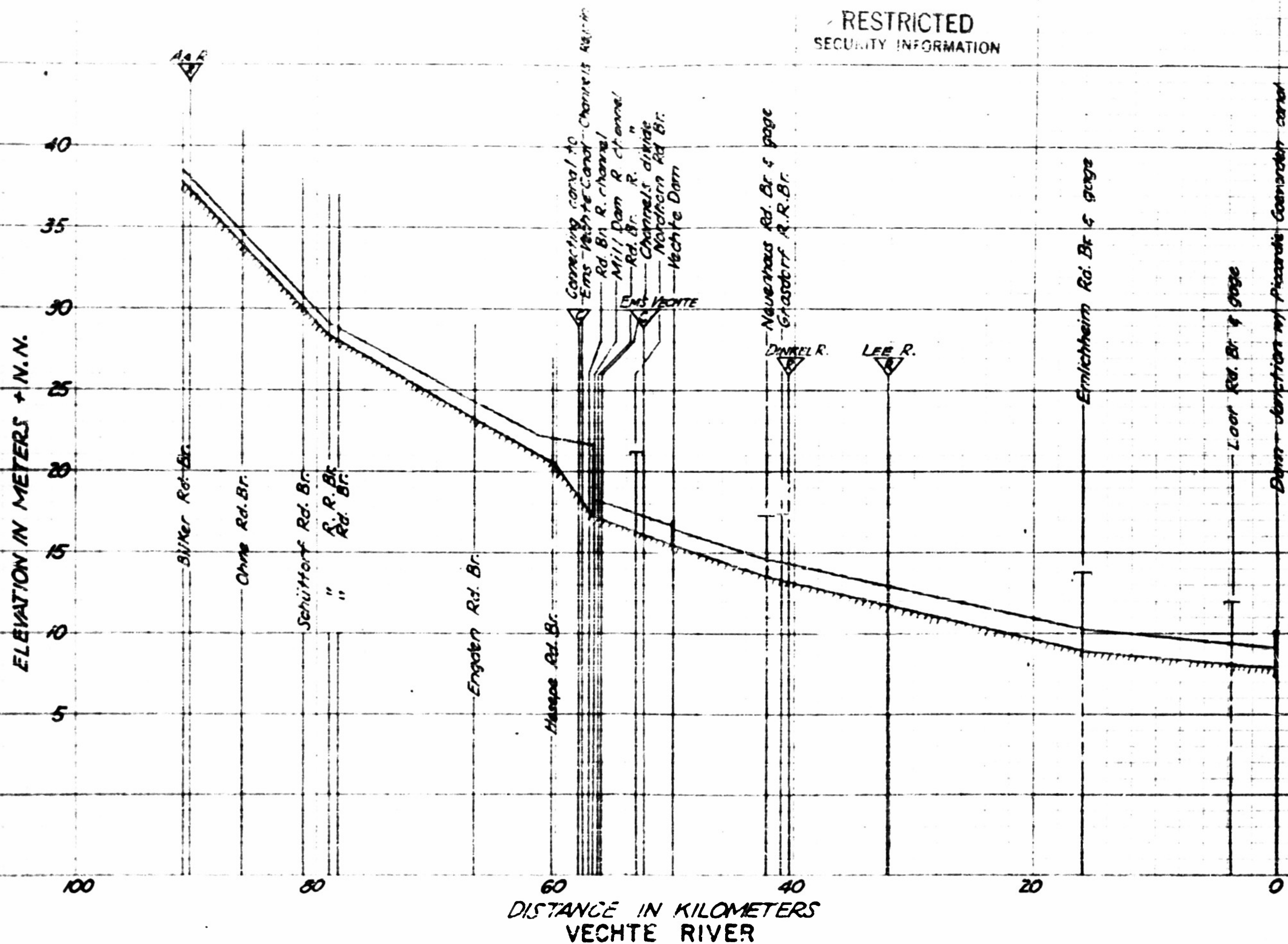


EMS RIVER
PROFILES
SUD-NORD CANAL
EMS-VECHTE CANAL

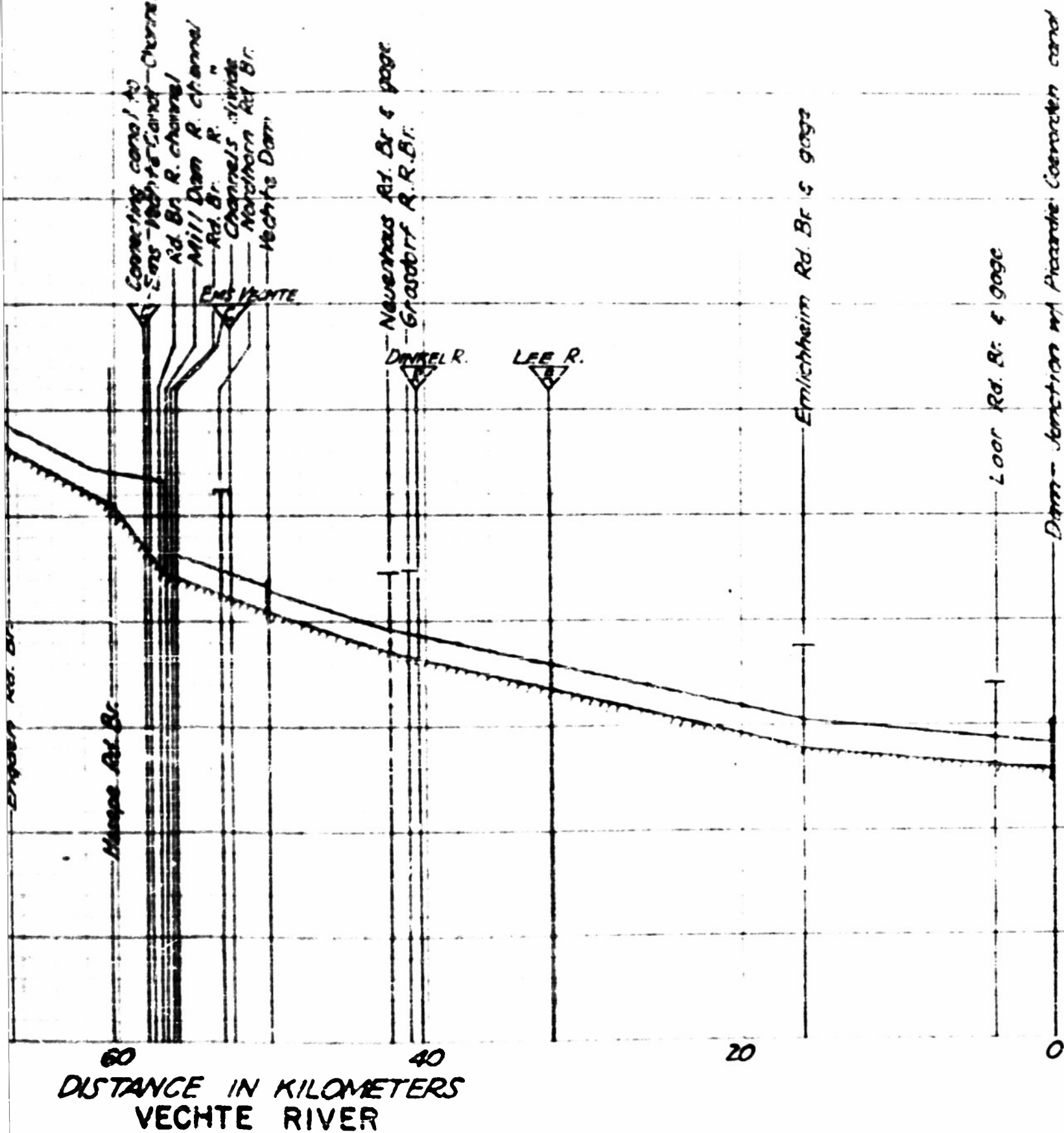
WASHINGTON DISTRICT COMPS OF ENGINEERS
Prepared by KE Date Aug-52
Drawn by KE

PLATE 8

RESTRICTED
SECURITY INFORMATION



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SECURITY INFORMATION



LEGEND

△ River junction ▽ Canal junction

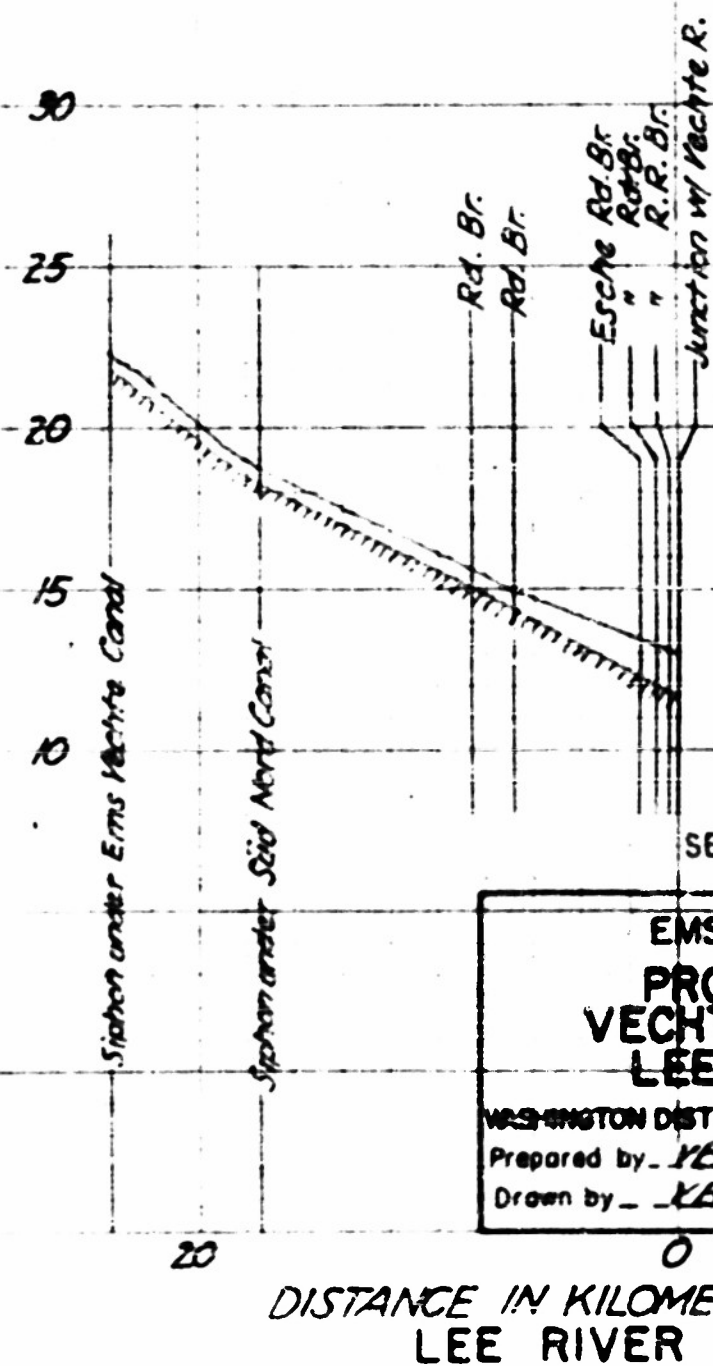
— Bridge showing clearance

--- Gage

— Dam or Arch

--- Average channel bottom

— Mean water



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EMS RIVER
PROFILES
VECHTE RIVER
LEE RIVER

WASHINGTON DISTRICT CORPS OF ENGINEERS

Prepared by - *YB* - Date *Aug 52*

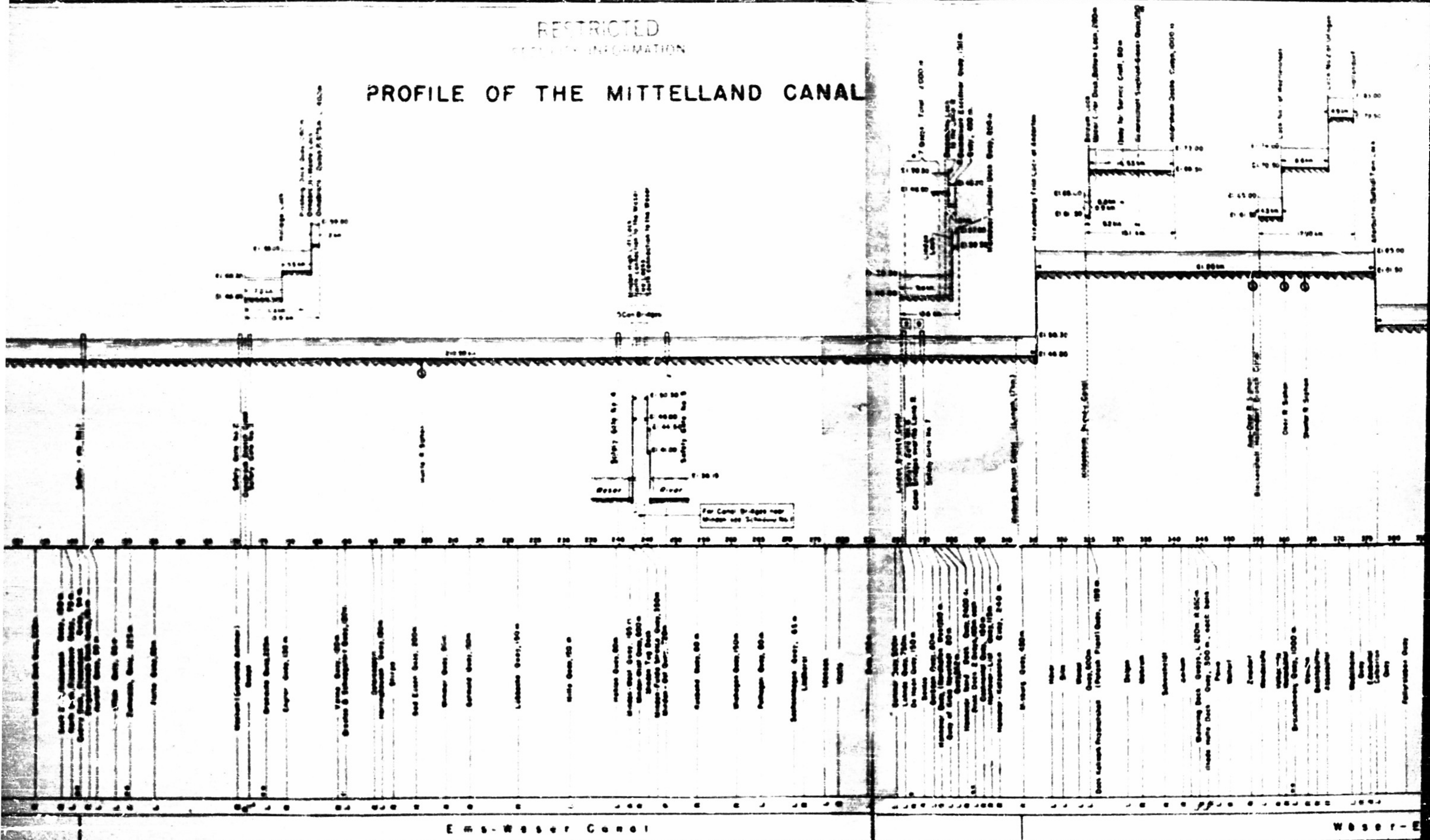
Drawn by - *YB* -

PLATE

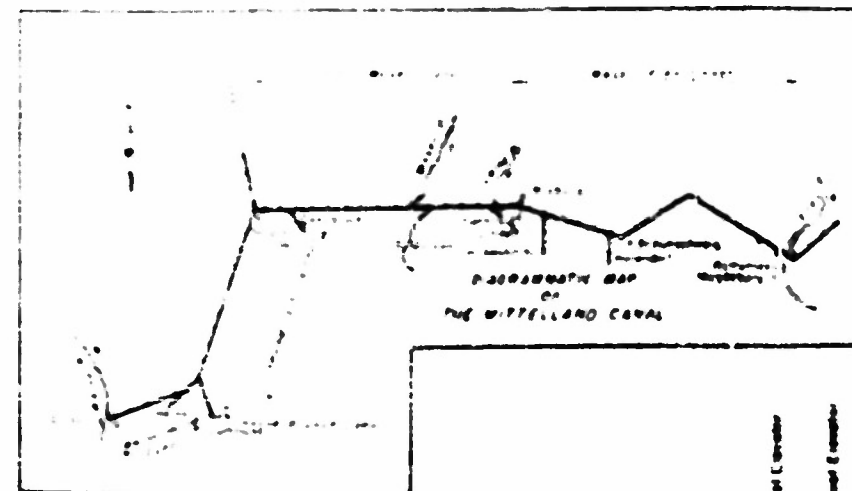


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SECURITY INFORMATION

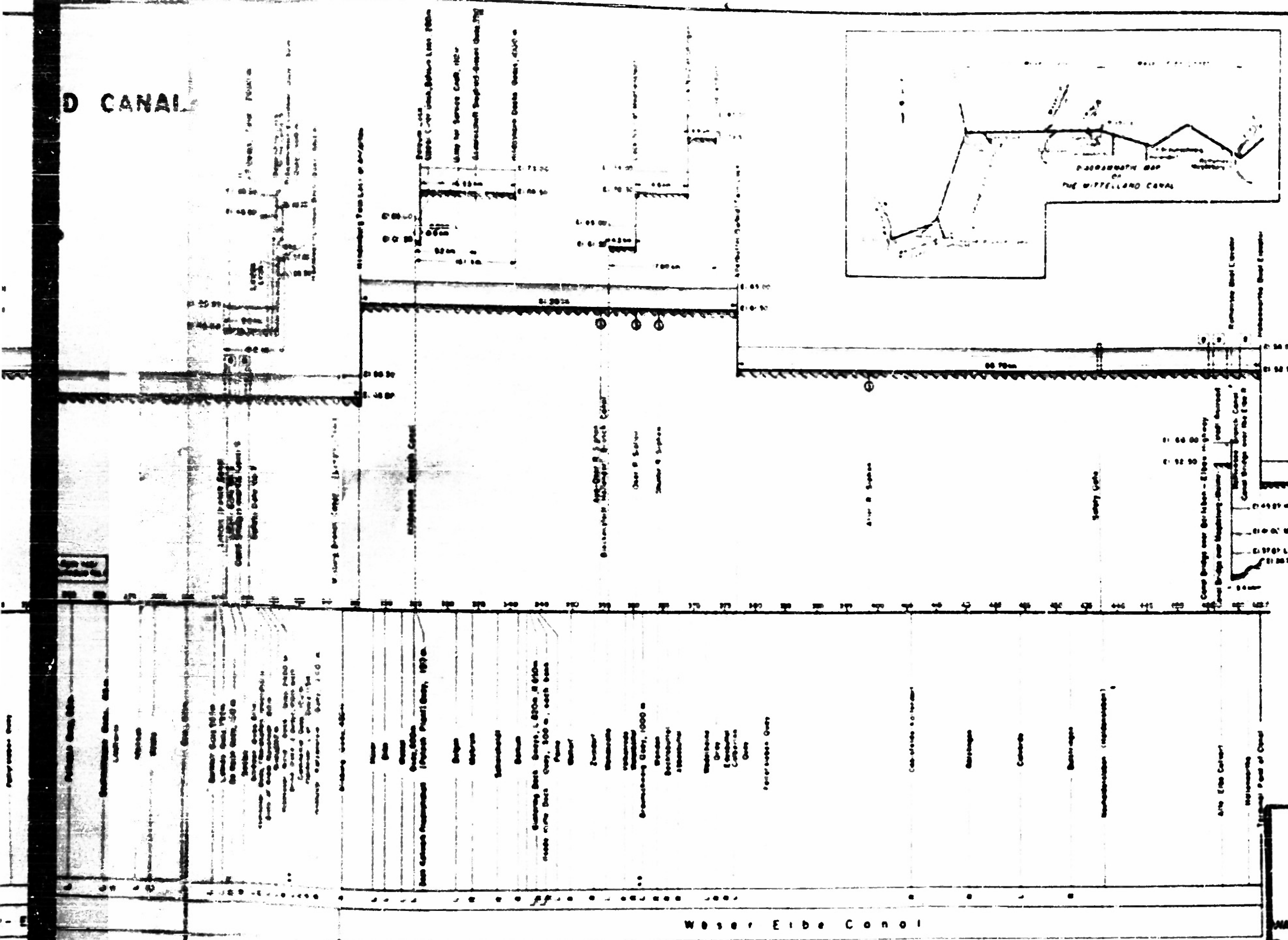
PROFILE OF THE MITTELLAND CANAL



D CANAL



Reproduced from
"NAVIGABLE WATER-
WAYS OF GERMANY"
S.E.S. 128, O.C.E.
U.S.A., August,
1944.



SCHEDULE NO. 1
Canal Bridges near Elsdon

Sta. No.	Canal Bridges
2437	Canal Bridge over Workhouse
2438	Canal Bridge over the Wood
2443	Canal Bridge over Friedrich Wilhelm
2445	Canal Bridge over Bismarck-Memorial
2471	Canal Bridge over Bismarck Memorial

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EMS RIVER PROFILE MITTELLAND CANAL

WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by *J. J. H.* Date *Aug 1952*
Drawn by *---*

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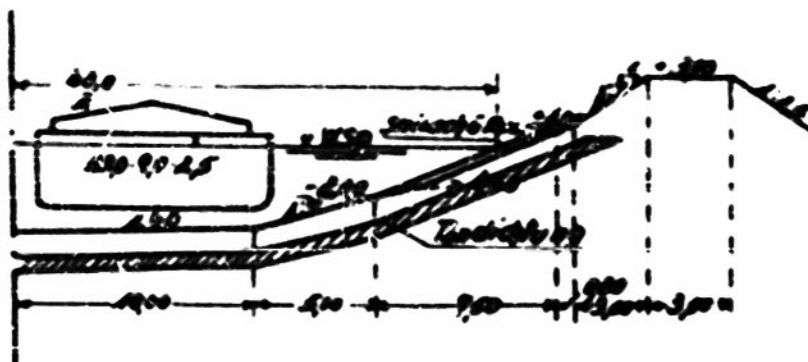


Fig. 13-2
Cross Section of Water Prism of the Reconstructed
Dortmund-Ems Canal
V-802 p.93 (1930)



Fig. 13-3
Typical Cross Section of Water Prism of the Reconstructed
Dortmund-Ems Canal

One of the banks is protected by steel sheet piling
to reduce the volume of excavation.
V-481 p.120 (1934)

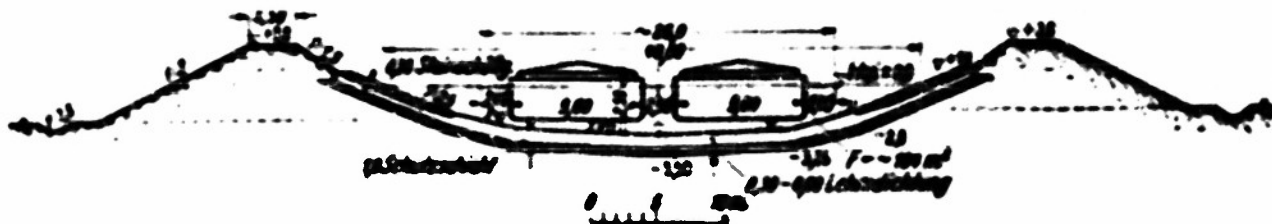


Fig. 13-4
Typical Cross Section of a New By-Pass Canal
Dortmund-Ems Canal

Lehmdichtung: Clay lining
Schuttschicht: Protective layer
Steinschüttung: Broken stone

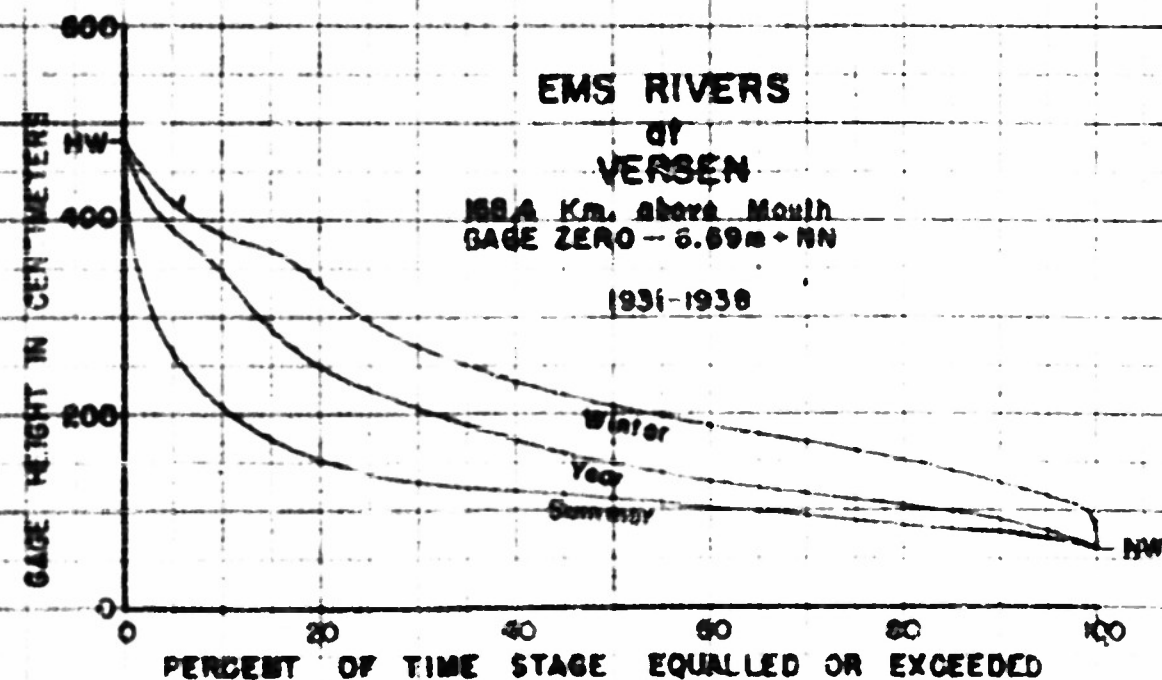
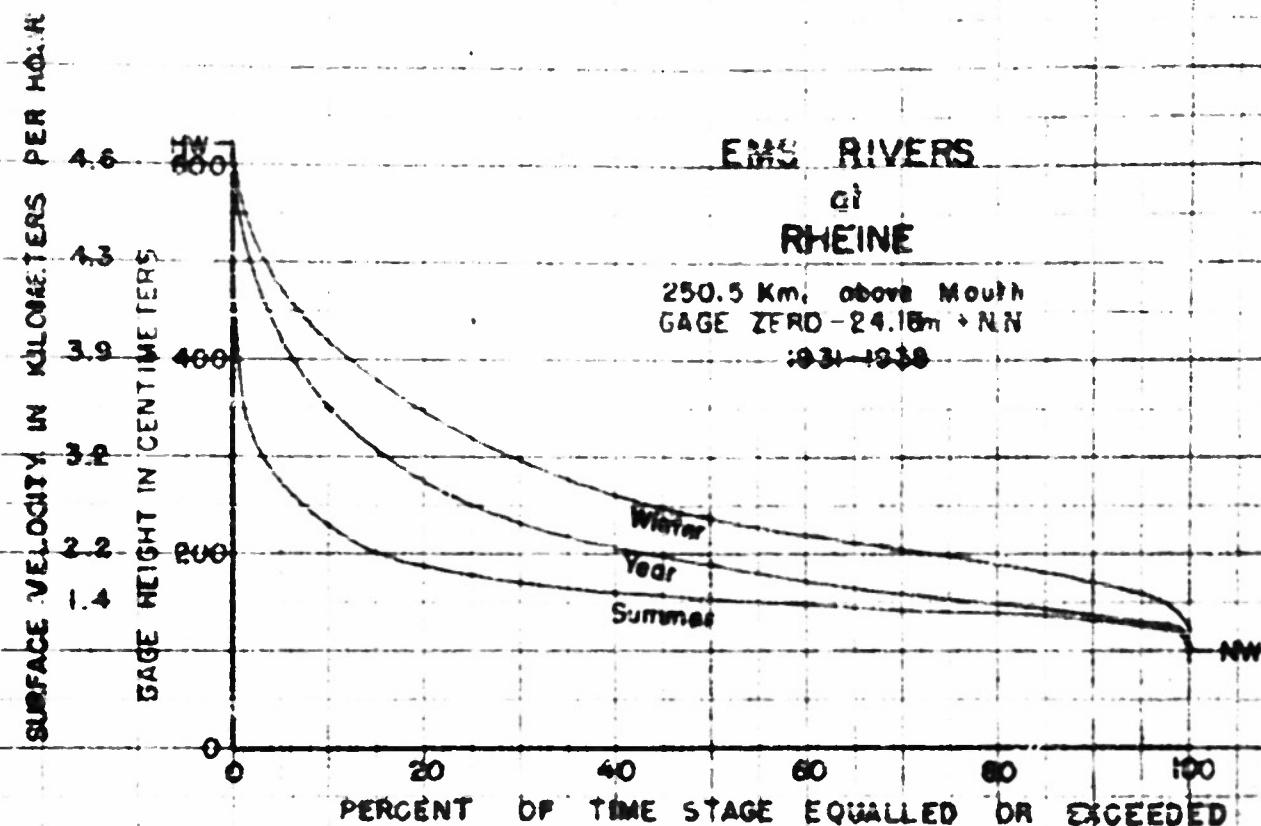
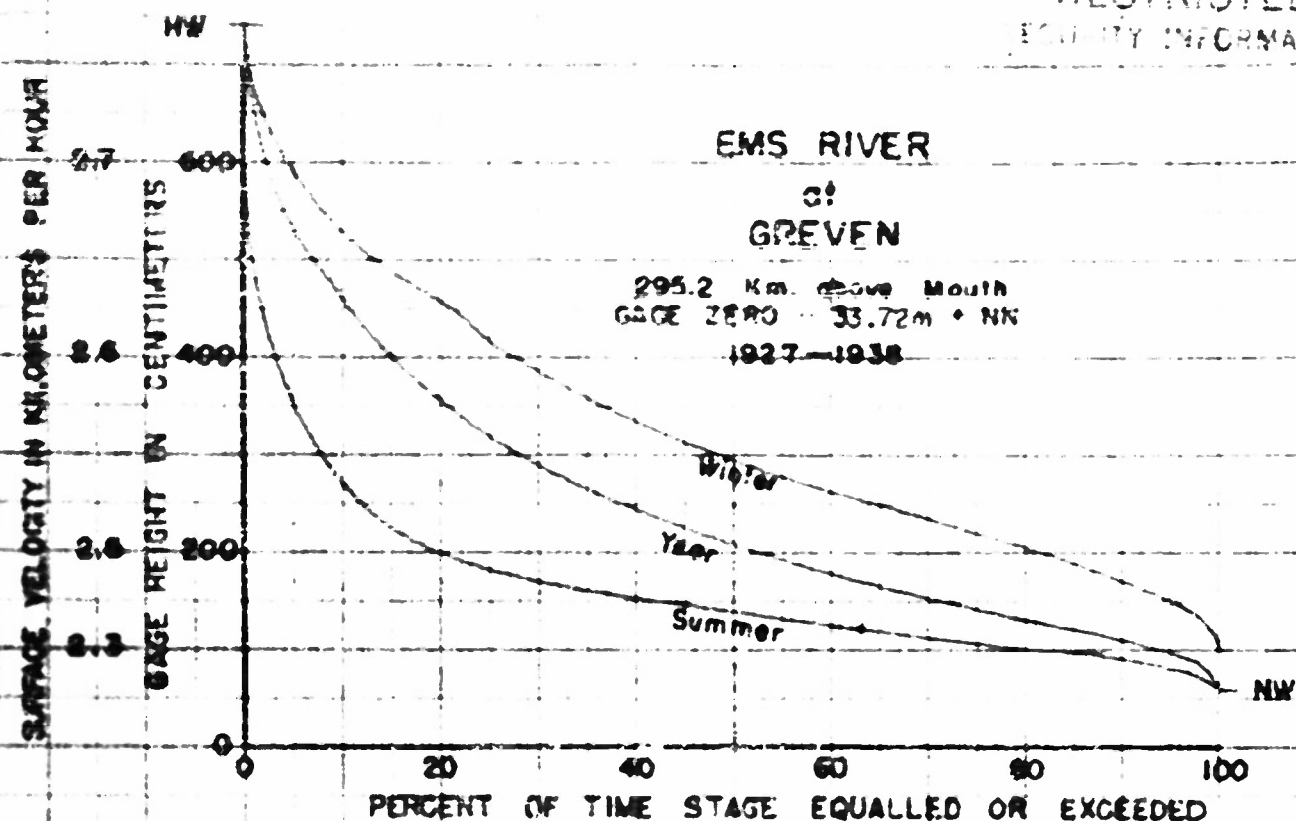
REPRODUCED FROM "NAVIGABLE WATERWAYS OF
GERMANY" S.E.S. 122, O.C.N., U.S.A.,
ATWENT, 1944.

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EMS RIVER	
CROSS SECTIONS	
DORTMUND EMS CANAL	
WASHINGTON DISTRICT CORPS OF ENGINEERS	
Prepared by <i>J.E.H.</i>	Date <i>Aug. 1952</i>
Drawn by _____	

PLATE II

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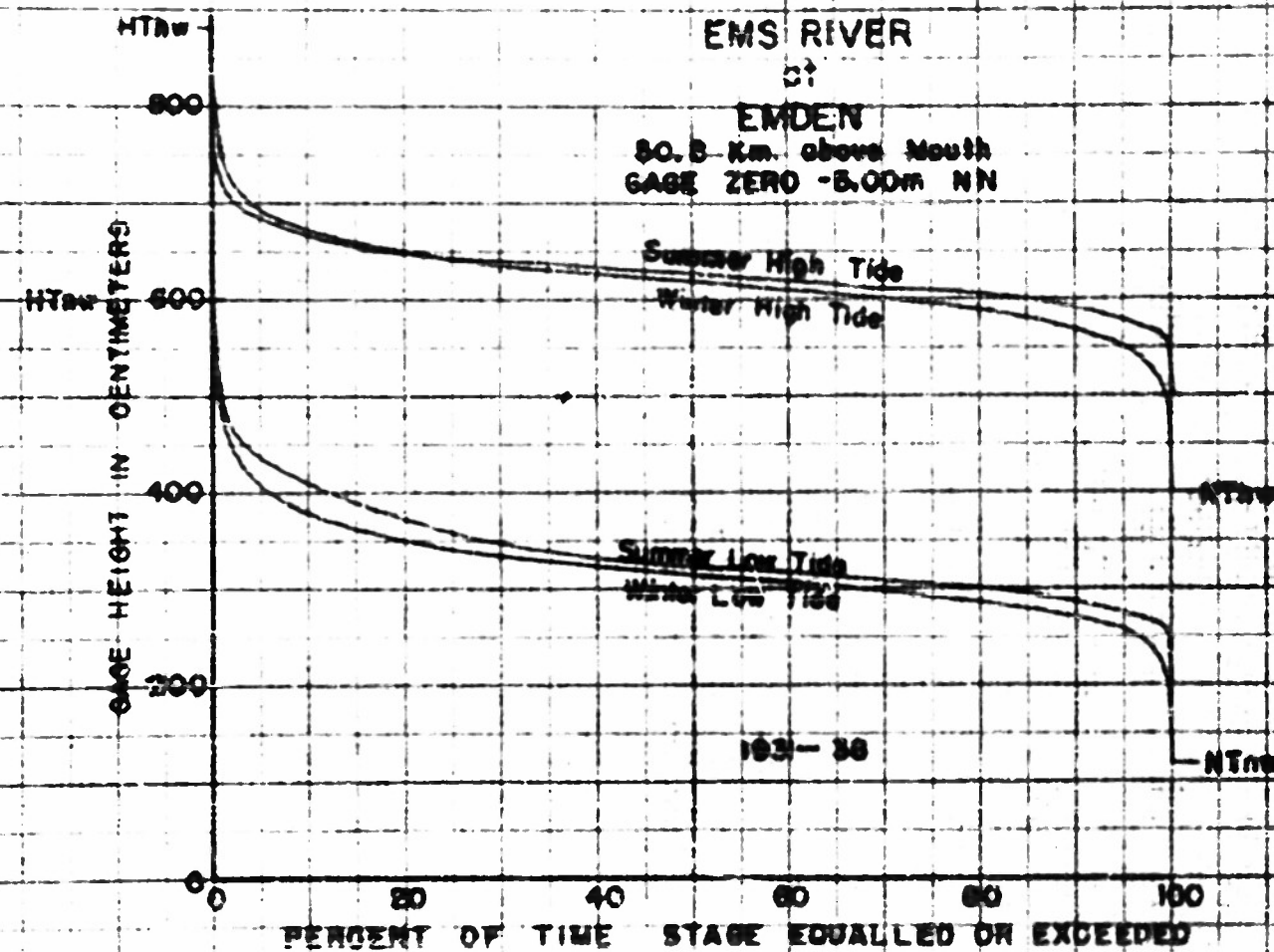
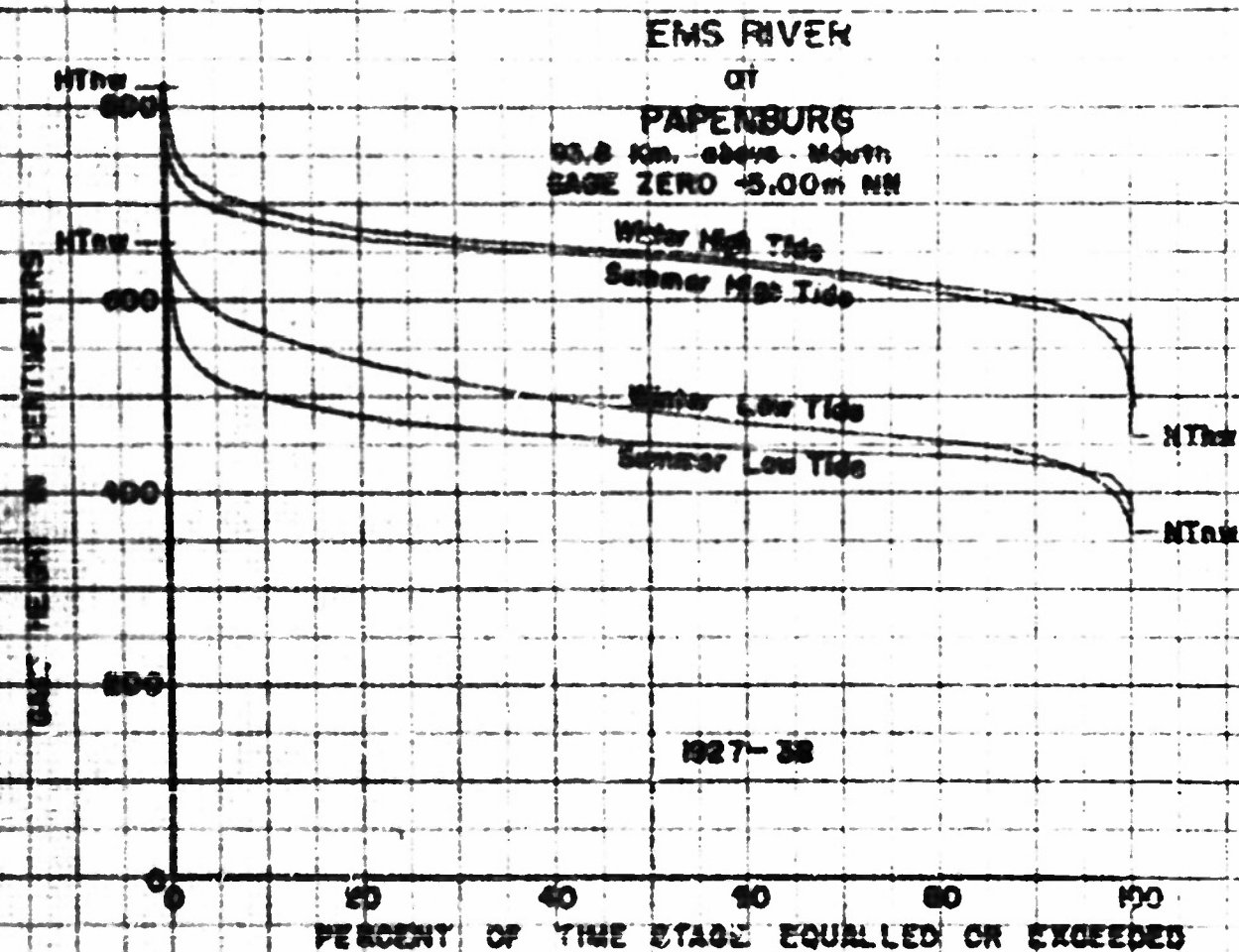


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SECURITY INFORMATION

EMS RIVER
STAGE DURATION
CURVES

WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by J.B. --- Date Aug 1952
Drawn by L.L.L. ---

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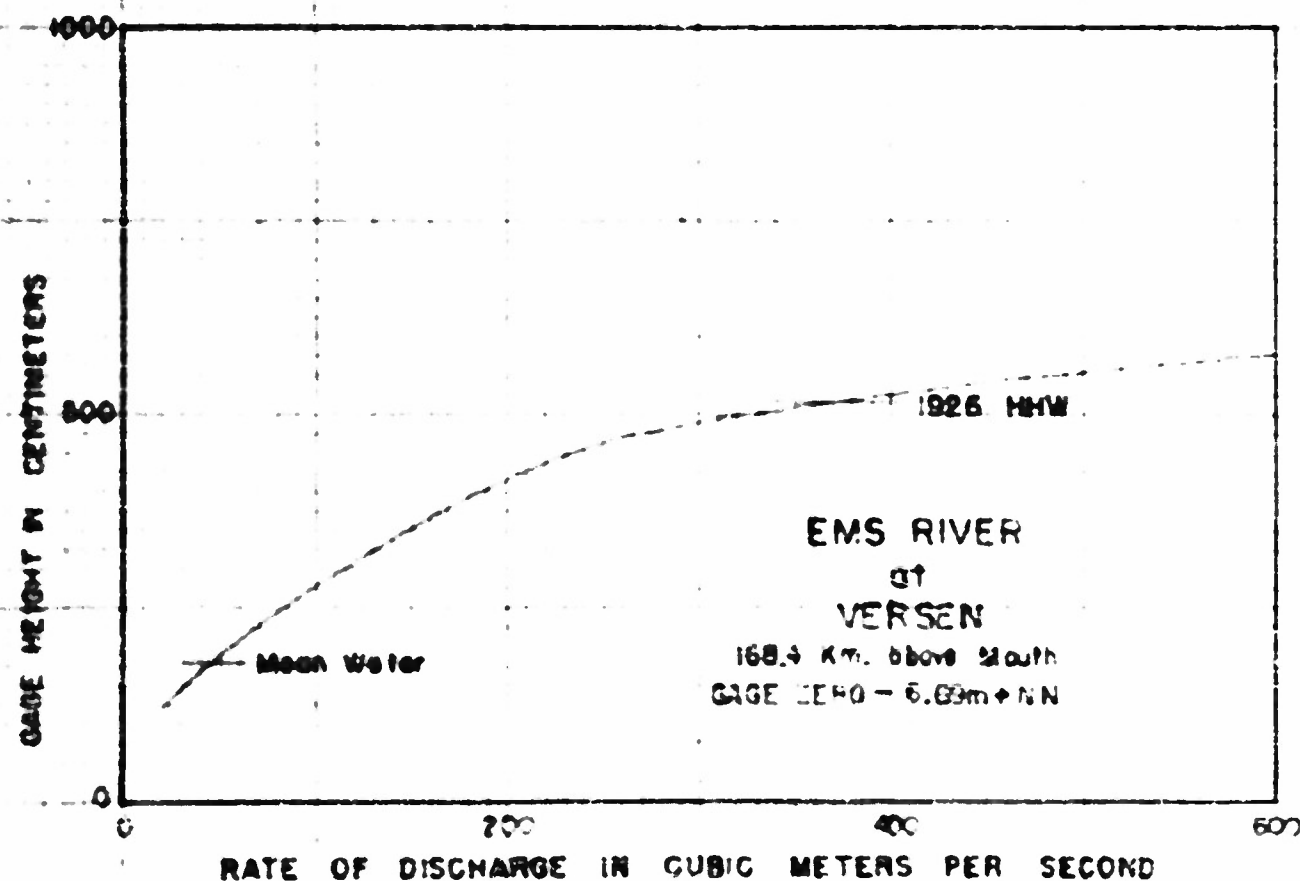
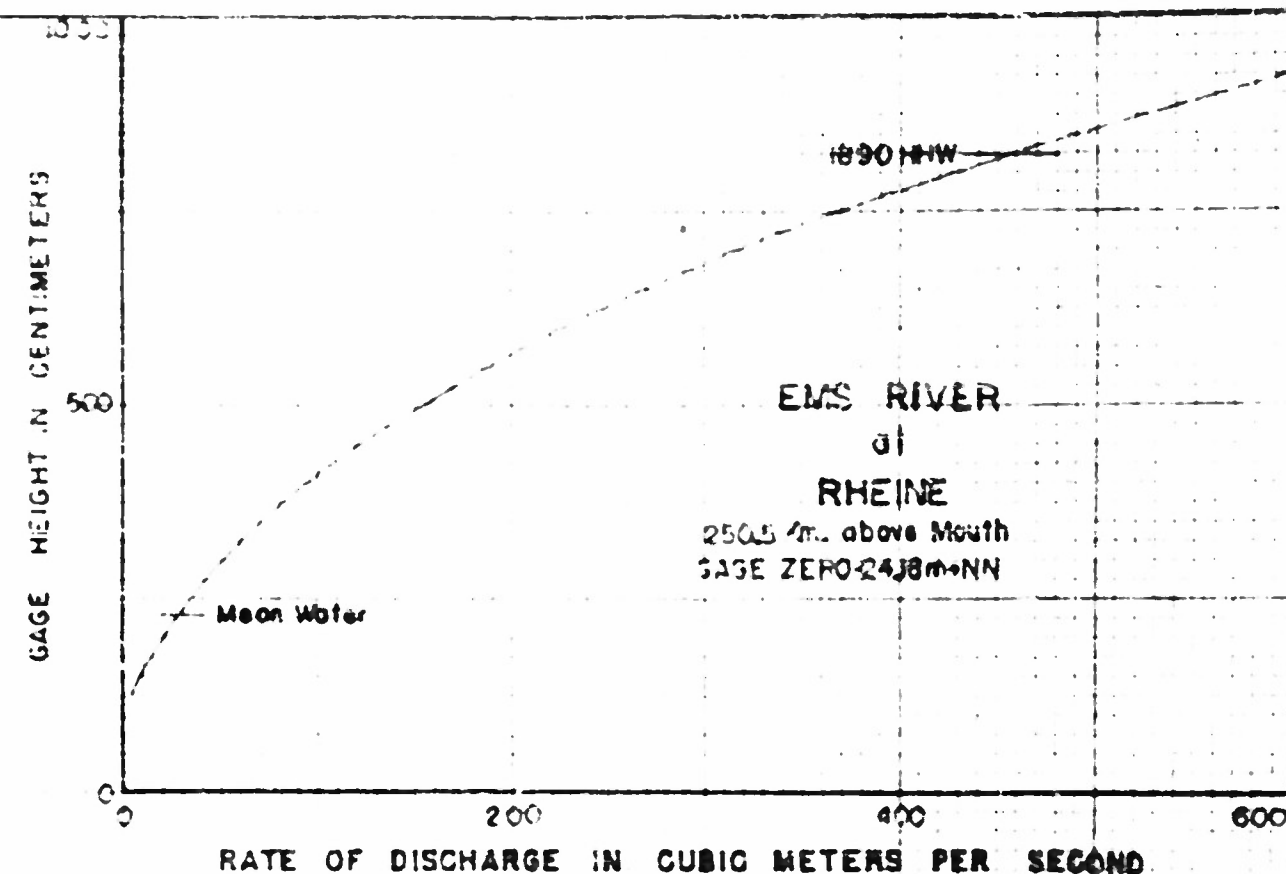
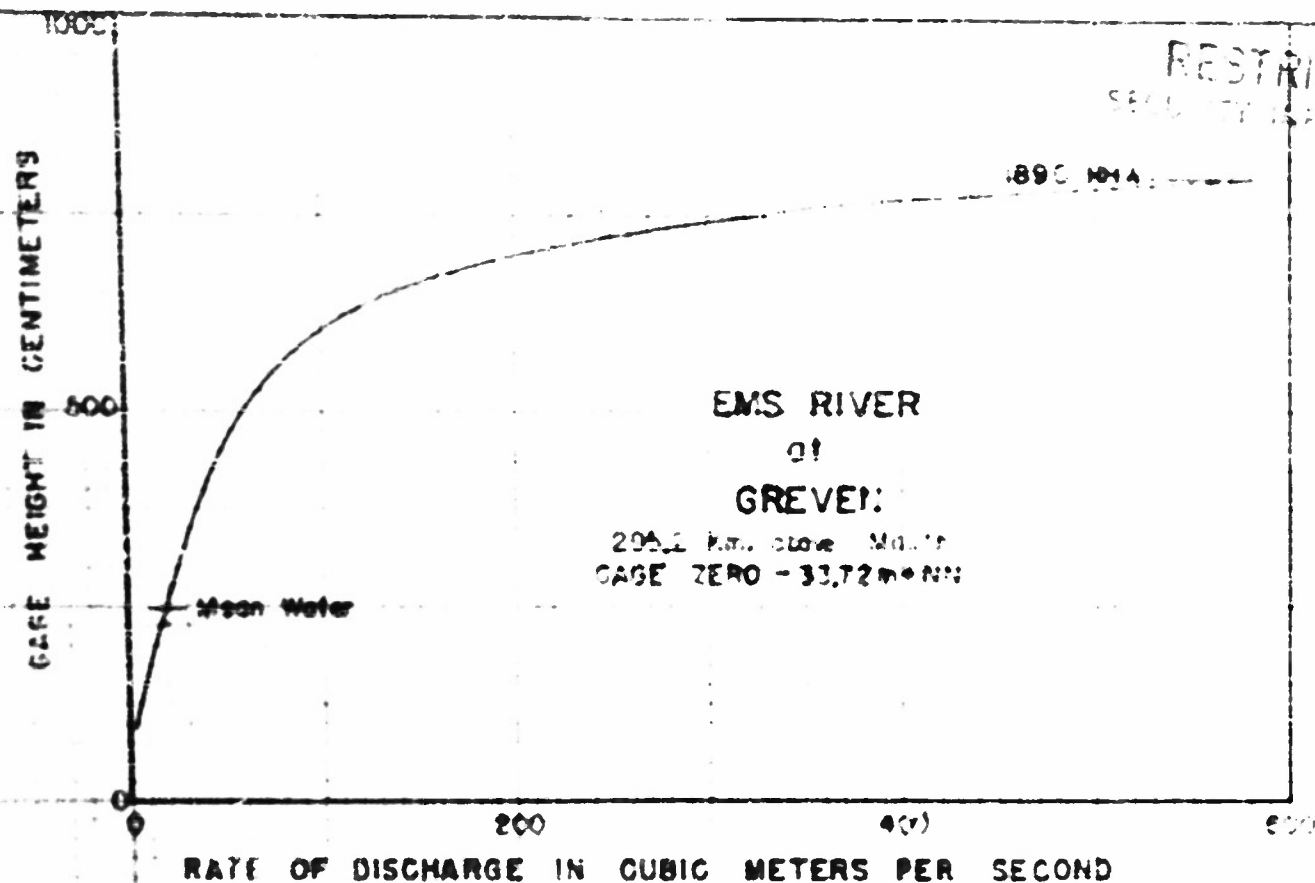


- NOTE: 1. Duration curves for entire year would be approximately midway between the corresponding seasonal curves.
2. See Table 2 for definition of abbreviations.

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EMS RIVER
STAGE DURATION
CURVES

WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by J.A. -- Date 11/15/38
Drawn by J.H. --



SOURCE: Rating curves estimated from data in Jahrbuch für die Gewässerkunde des Deutschen Reichs, 1937-38 B in Jahrbuch für die Gewässerkunde Norddeutschlands, 1927-36.

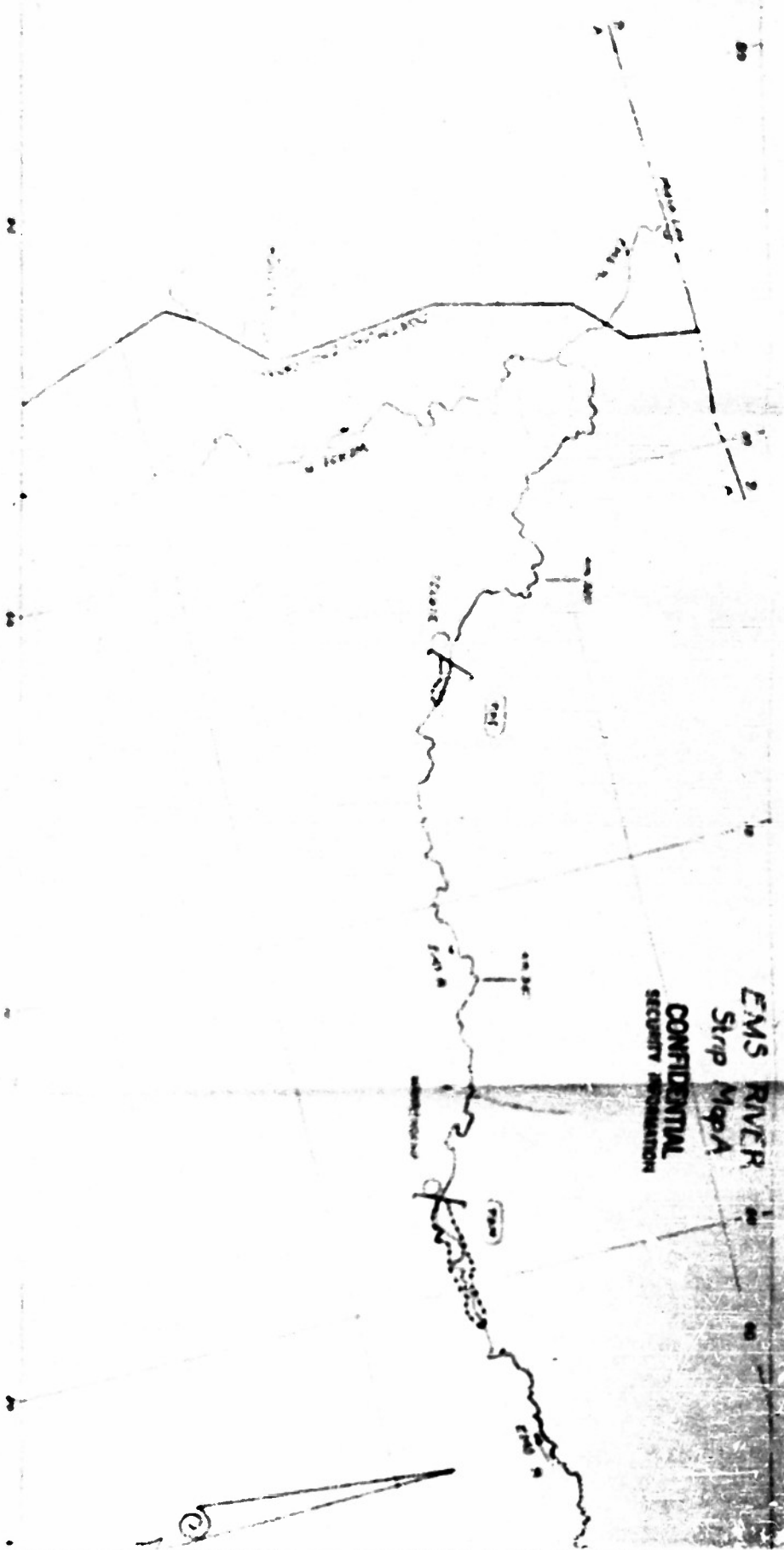
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EMS RIVER
DISCHARGE RATING
CURVES

WASHINGTON DISTRICT CORPS OF ENGINEERS
Prepared by F.S.R. Date AUG. 1952
Drawn by J.H.

PLATE

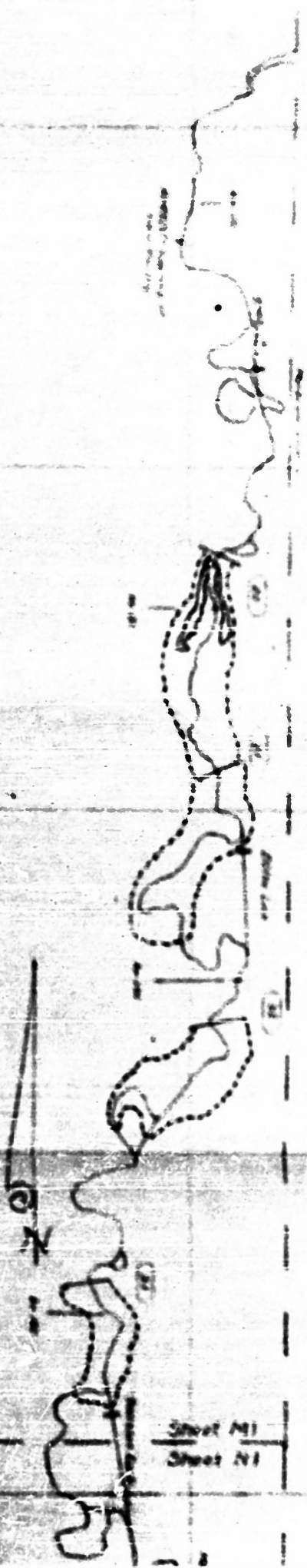
EMS RIVER
Strip Map A
CONFIDENTIAL
SECURITY INFORMATION

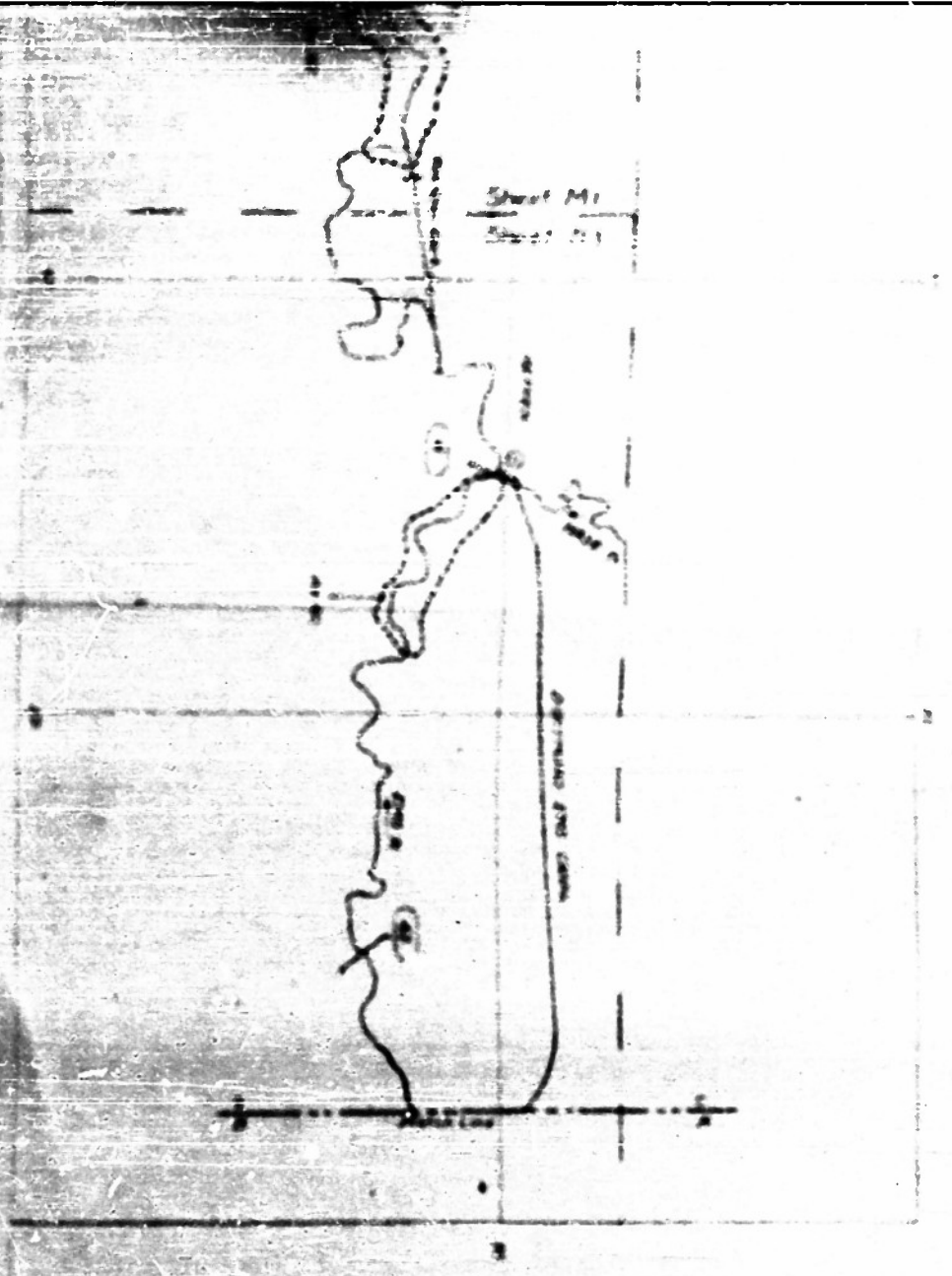
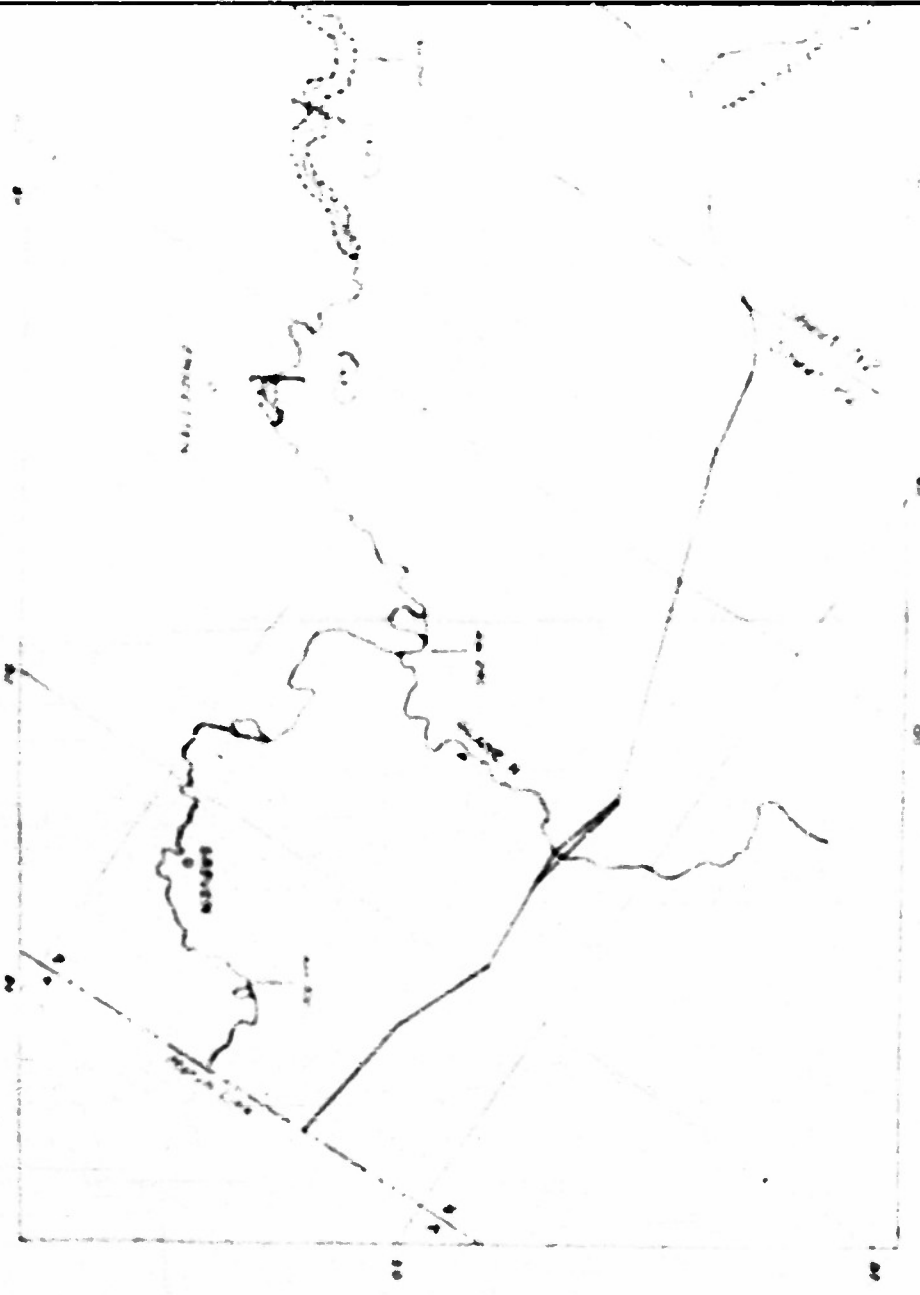
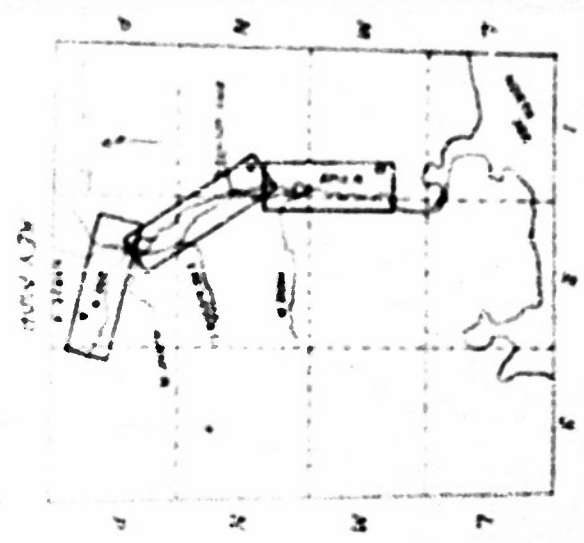


EMS RIVER
Strip Map B



EMS RIVER
Strip Map C





LEGEND

- 1. Shore line (dashed line) (see notes)
- 2. Main line or Ship Moors
- 3. Moors or Landing Grounds
- 4. Buildings, Ship Landings
- 5. Low Water mark (see notes)
- 6. High Water or Flood (see notes)
- 7. Sea Level (see notes)



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 DOWNGRADING AND
 DECLASSIFICATION

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EXHIBIT A
DESCRIPTION OF BRIDGES AND DAMS
ELS RIVER.

Abstracted and Reproduced from "Report on R. Ems (Second Edition,"
Part II.) Main Hq. 21 Army Group, December 1944.

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Description of Bridges, Locks, Dams, etc.

NOTE

1. Accuracy. The majority of constructional details and measurements have been taken from documentary sources and unless otherwise stated have been checked and supplemented from air photos. Measurements of river widths refer to normal water levels.
2. Overriding not shown on GDS 1:46. A note "not marked on map" has been included in the remarks column wherever a road or railway bridge exists which does NOT appear on the current edition of the sheet concerned in the GDS 1:46 series. It has not been thought necessary to do so in the case of farm bridges, footbridges and other constructions.

3. Abbreviations

L. U.	Low water.
M. U.	Mean low water
H. U.	Mean high water
H. W.	High water
H. N. 2.	Highest navigable water
H. N.	Normal
H. N. (G.D.S.)	Normal (G.D.S.)

Load classification of bridges. The load classification of bridges, as given in this schedule is almost entirely taken from official German ratings by conversion according to the following table:-

GERMAN CLASS	MEANING	BRITISH EQUIVALENT
I	2-axled vehicles up to class 40 or over.	
II	24 (metric) tons simply and axled.	
III	" " up to 16 tons. class 24.	
IV	" " up to 7 tons. class 12.	
	" " less than 7 tons. class 12.	

(b) Railway bridges

N.	max axle load 25 tons.
E.	" " 25 "
G.	" " 20 "
H.	" " 10 "
J.	" " 16 "
K.	" " less than 16 tons.

Where the German sources give only a civil rating in tons, usually in the case of viaducts, this has been recorded in the schedule as e.g. "3 tons". This civil figure is the heaviest axle load in metric tons for 2-axled vehicles without trailers and gives a guide to the strength of the bridge until the military classification can be ascertained. It will be noted that values for in this schedule include bridges of greater capacity.

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AS THE BRIDGE IS OUT

DESCRIPTION OF BRIDGES, DAMS, ETC.

GENERAL DATA

Section	Type	Location	Map Ref	Construction	Details	Notes
1	Bridge	1.0000	1.0000	1.0000	1.0000	1.0000
2	Bridge	2.0000	2.0000	2.0000	2.0000	2.0000
3	Bridge	3.0000	3.0000	3.0000	3.0000	3.0000
4	Bridge	4.0000	4.0000	4.0000	4.0000	4.0000
5	Bridge	5.0000	5.0000	5.0000	5.0000	5.0000
6	Bridge	6.0000	6.0000	6.0000	6.0000	6.0000
7	Bridge	7.0000	7.0000	7.0000	7.0000	7.0000
8	Bridge	8.0000	8.0000	8.0000	8.0000	8.0000
9	Bridge	9.0000	9.0000	9.0000	9.0000	9.0000
10	Bridge	10.0000	10.0000	10.0000	10.0000	10.0000
11	Bridge	11.0000	11.0000	11.0000	11.0000	11.0000
12	Bridge	12.0000	12.0000	12.0000	12.0000	12.0000
13	Bridge	13.0000	13.0000	13.0000	13.0000	13.0000
14	Bridge	14.0000	14.0000	14.0000	14.0000	14.0000
15	Bridge	15.0000	15.0000	15.0000	15.0000	15.0000
16	Bridge	16.0000	16.0000	16.0000	16.0000	16.0000
17	Bridge	17.0000	17.0000	17.0000	17.0000	17.0000
18	Bridge	18.0000	18.0000	18.0000	18.0000	18.0000
19	Bridge	19.0000	19.0000	19.0000	19.0000	19.0000
20	Bridge	20.0000	20.0000	20.0000	20.0000	20.0000
21	Bridge	21.0000	21.0000	21.0000	21.0000	21.0000
22	Bridge	22.0000	22.0000	22.0000	22.0000	22.0000
23	Bridge	23.0000	23.0000	23.0000	23.0000	23.0000
24	Bridge	24.0000	24.0000	24.0000	24.0000	24.0000
25	Bridge	25.0000	25.0000	25.0000	25.0000	25.0000
26	Bridge	26.0000	26.0000	26.0000	26.0000	26.0000
27	Bridge	27.0000	27.0000	27.0000	27.0000	27.0000
28	Bridge	28.0000	28.0000	28.0000	28.0000	28.0000
29	Bridge	29.0000	29.0000	29.0000	29.0000	29.0000
30	Bridge	30.0000	30.0000	30.0000	30.0000	30.0000
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36	Bridge	36.0000	36.0000	36.0000	36.0000	36.0000
37	Bridge	37.0000	37.0000	37.0000	37.0000	37.0000
38	Bridge	38.0000	38.0000	38.0000	38.0000	38.0000
39	Bridge	39.0000	39.0000	39.0000	39.0000	39.0000
40	Bridge	40.0000	40.0000	40.0000	40.0000	40.0000
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71	Bridge	71.0000	71.0000	71.0000	71.0000	71.0000
72	Bridge	72.0000	72.0000	72.0000	72.0000	72.0000
73	Bridge	73.0000	73.0000	73.0000	73.0000	73.0000
74	Bridge	74.0000	74.0000	74.0000	74.0000	74.0000
75	Bridge	75.0000	75.0000	75.0000	75.0000	75.0000
76	Bridge	76.0000	76.0000	76.0000	76.0000	76.0000
77	Bridge	77.0000	77.0000	77.0000	77.0000	77.0000
78	Bridge	78.0000	78.0000	78.0000	78.0000	78.0000
79	Bridge	79.0000	79.0000	79.0000	79.0000	79.0000
80	Bridge	80.0000	80.0000	80.0000	80.0000	80.0000
81	Bridge	81.0000	81.0000	81.0000	81.0000	81.0000
82	Bridge	82.0000	82.0000	82.0000	82.0000	82.0000
83	Bridge	83.0000	83.0000	83.0000	83.0000	83.0000
84	Bridge	84.0000	84.0000	84.0000	84.0000	84.0000
85	Bridge	85.0000	85.0000	85.0000	85.0000	85.0000
86	Bridge	86.0000	86.0000	86.0000	86.0000	86.0000
87	Bridge	87.0000	87.0000	87.0000	87.0000	87.0000
88	Bridge	88.0000	88.0000	88.0000	88.0000	88.0000
89	Bridge	89.0000	89.0000	89.0000	89.0000	89.0000
90	Bridge	90.0000	90.0000	90.0000	90.0000	90.0000
91	Bridge	91.0000	91.0000	91.0000	91.0000	91.0000
92	Bridge	92.0000	92.0000	92.0000	92.0000	92.0000
93	Bridge	93.0000	93.0000	93.0000	93.0000	93.0000
94	Bridge	94.0000	94.0000	94.0000	94.0000	94.0000
95	Bridge	95.0000	95.0000	95.0000	95.0000	95.0000
96	Bridge	96.0000	96.0000	96.0000	96.0000	96.0000
97	Bridge	97.0000	97.0000	97.0000	97.0000	97.0000
98	Bridge	98.0000	98.0000	98.0000	98.0000	98.0000
99	Bridge	99.0000	99.0000	99.0000	99.0000	99.0000
100	Bridge	100.0000	100.0000	100.0000	100.0000	100.0000

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Report on the (Name of Bridge), Description of bridge, etc., etc.

Serial	Type	Location & Name	Map Ref 1/100,000	Construction Details	Overall Length	Span Lengths	Width at Bridge	Notes
1.	Passenger Ferry	Water	750	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
2.	•	Water	7500	Small stone girder through bridge, 8 spans (175, 25, 25, 2 - 205, 150, 150, 155 ft clear spans left to right). The two center spans form a swinging bridge on the pier between, which is prevented by timber piles. The right spans and one left span are land spans. Piers and abutments concrete.	1075	25	100 20.5 10	Notes elsewhere see p. 1
3.	Passenger Ferry	Water	7500	Just upstream at 7500 on 8 spans steel joist and timber truss bridge joining the island to the left bank across a 200 ft wide channel.	1075	25	100 20.5 10	Notes elsewhere see p. 1
4.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
5.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
6.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
7.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
8.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
9.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1
10.	Passenger Ferry	Water	7500	Passenger Ferry probably running from small harbor in 1940-1941. One channel 100 yds long by 20 to 30 ft wide to river and crosses to right bank at 7500 yds. Unimproved track leads for 400 yds over station to Quarantine - 1000 ft.	1075	25	100 20.5 10	Notes elsewhere see p. 1

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GENERAL INFORMATION				ALL INFORMATION IS TRUE				Page 3/6	
NO.	TYPE	LOCATION & DATE	DESCRIPTION OF PROJECT, AND ETC.	CONSTRUCTION DETAILS	Overall Length	Width	Load Class	Steel Deck Depth	Remarks
1	Beam Bridge	1/10/60	See notes	The bridge is parallel. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
2	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
3	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
4	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
5	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
6	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
7	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
8	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
9	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
10	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
11	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water
12	Beam Bridge	1/10/60	See notes	Bridge over lower end of old and new locks in series. Old low 142 to 160 ft. new low 140 to 160 ft. Each of old low 142 ft. (natural timber) but wide. The new low is on the right. Full of 40 ft. concrete north at normal level.	90	104 2 x 1.2	10	10 1/2	about 10 1/2 ft. at low water

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REPORT ON R. AND (UNCLASSIFIED) Description of bridges, etc.

ALL MEASUREMENTS IN FEET

Page 30

Serial	Type	Location & Name	Map Ref 1:100,000	Construction Details	Overall Length	Span	Abutments	Overall	Load Class	After Repair Span L x B, H x C	Notes
41	road bridge	REYES	68000	6 span steel girder, with timber roadway. Clear span 4 x 15.5, 2 x 20.5. Probably replaced by 4 span reinforced concrete bridge attributed for construction in 1937	280	13.7	1 x 1.6	28	197	28	
51	road bridge	REYES	about 68151	On left approach 2.6 line from bridge serial 41 3 span reinforced concrete, clear spans 2 x 30 1 x 5 ft	182	19.7	1 x 10.5	as listed 10			
61	road bridge	REYES	about 67945	On left approach 2.6 line from bridge serial 41 2 span reinforced concrete, clear spans 2 x 16.5 roughly installed approach roads no foot visible.	127	10		28	18		
71	road bridge	REYES	about 68057	On left approach 2.6 line from bridge serial 41 2 span reinforced concrete, clear spans 2 x 20.7, 5 x 25.5, 1 x 21.5 ft.	209	12.4	1 x 1.3	18	28		
81	road bridge	REYES	67940	No bridge or ferry exists here							
91	road bridge	REYES	68090	5 span reinforced concrete, each span 47.6 ft	209	12.4	1 x 1.3	18	28		
101	road bridge	REYES	69157		185	10		18	28		
111	road bridge	REYES	69735	6 of 7 span 7 steel girder and timber bridge	214	12.3	2 x 1.4	28	18		
121	road bridge	REYES	70532	Steel girder, 2 spans of 92 ft	214	12.3	2 x 1.4	28	18		
131	road bridge	REYES	70530		214	12.3	2 x 1.4	28	18		
141	road bridge	REYES	71134	3 span steel girder through bridge. Clear spans 53.6, 18, 197 ft, double track fly, double approach.	254	14.3	2 x 1.4	28	18		
151	road bridge	REYES	72531	4 span reinforced concrete, beam spans 2 x 18, 2 x 19. Double approach 7 roadway approaches or piers.	260	14.3	2 x 1.4	28	18		

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SECURITY INFORMATION

REPORT 1. 25 (2001-21121) Security Status of Bridges, Dams, etc.

ALL INFORMATION IS FURT

Page 20

Serial	Type	Location	No. of Structures	Construction Details	Overall Length Feet	Overall Width Feet	Overall Height Feet	Overall Area Sq. Ft.	Overall Volume Cu. Yd.	Overall Weight Tons	Overall Value \$	Overall Risk Level
13	Load Bridges	BRIDGE	100-1	3 parallel spans over river at 117. 118. 119. 120. 121. 122. 123. 124. 125. 126. 127. 128. 129. 130. 131. 132. 133. 134. 135. 136. 137. 138. 139. 140. 141. 142. 143. 144. 145. 146. 147. 148. 149. 150. 151. 152. 153. 154. 155. 156. 157. 158. 159. 160. 161. 162. 163. 164. 165. 166. 167. 168. 169. 170. 171. 172. 173. 174. 175. 176. 177. 178. 179. 180. 181. 182. 183. 184. 185. 186. 187. 188. 189. 190. 191. 192. 193. 194. 195. 196. 197. 198. 199. 200. 201. 202. 203. 204. 205. 206. 207. 208. 209. 210. 211. 212. 213. 214. 215. 216. 217. 218. 219. 220. 221. 222. 223. 224. 225. 226. 227. 228. 229. 230. 231. 232. 233. 234. 235. 236. 237. 238. 239. 240. 241. 242. 243. 244. 245. 246. 247. 248. 249. 250. 251. 252. 253. 254. 255. 256. 257. 258. 259. 260. 261. 262. 263. 264. 265. 266. 267. 268. 269. 270. 271. 272. 273. 274. 275. 276. 277. 278. 279. 280. 281. 282. 283. 284. 285. 286. 287. 288. 289. 290. 291. 292. 293. 294. 295. 296. 297. 298. 299. 300. 301. 302. 303. 304. 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• The following table shows the number of people who have been convicted of a crime in the United States since 1970. The data is presented in millions of people.

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REPORT 0 - 010 (MILITARY) DISTRIBUTION OF BRIDGES, AND ETC.

Serial	Type	Location & Map Ref	Construction details	Overall length	Width	Span	Notes
74	Beam bridge	at of 010710	1 span 100 ft long on concrete abutments. The left floodlight span on south approach, and bridge over left side abutment is still in north approach.	100	10	35	
75	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
76	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
77	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
78	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
79	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
80	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
81	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
82	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
83	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
84	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
85	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
86	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
87	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
88	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
89	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	
90	Beam bridge	at of 010710	1 span 100 ft long. Approached from north.	100	10	35	

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EXHIBIT B
DESCRIPTION OF WATERCOURSE
AND CONTROL STRUCTURES*

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The Ems River	1
Dortmund-Ems Canal	5
Ems River, Left Side Canals	11

*Abstracted and Translated from "Stromgebiet der Weser und Ems, Einwirkung auf der Wasserfuhrung" (River Basins of the Weser and Ems, Influence of Flow). Military Geography ("Mil-Geo") Training Manual H. Dv. g. 33a, General Staff of German Army, Section 9, Berlin, 1937.

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THE EMS RIVER

I. Description of the watercourse.

Kilometering begins:

1. At the source (Hoeverhof/Senne)
2. At Groven, on the Schoeneflieth Dam
(Groven am Mehr Schoeneflieth)
(beginning of the official kilometering)
3. At Meppen-Koppe Lock (Meppen-Koppelschlouse)
(official river-kilometering in the lower course
and tidal area)

Navigation begins at Groven (river-km 0.0) up to river-km 71.8, for vessels of 80-ton capacity, from there up to Hanekenfaehr (river-km 84.4) at MW, for ships up to 150-ton capacity. For further information see "Dortmund-Ems Canal."

The river is often covered with ice up to 30 cm thick in winter; in many winters, only ground ice is formed. Navigation is discontinued on the average of 27 days each year due to ice and high water.

The drainage area of the Ems to Papenburg is 9,500 km². From this, the Hase River, the largest Ems tributary (see "Hase River"), contributes 3,125 km², and Grosse Aa River 930 km².

The precipitation from January 1925-1935 averaged 743 mm (mean average for Germany 690 mm, in North German plains 600 mm), which equals 7,430 m³ of rain per hectare. The yearly precipitation in the 347,000-hectare area of the Rhine District equals, therefore, 347,000 x 7,430 equals 2,780 mil. m³ which is 25 percent of the total precipitation in the Ems R. basin. Out of this flow, 1,047 mil. m³ equals 38 percent, flows into the Ems, and the remaining 62 percent is lost in seepage. Even though summer precipitation of 431 mm exceeds by 40 percent the winter precipitation of 312mm, the discharge volume during summer is only 1/3 of the winter volume, so that winter high water and flooding are considerably greater than summer high water.

To date, the highest winter high water in the Ems (Nov. 1890) was 460 m³/s, in the Hase at Herzlake, 190 m³/s (Jan. 1925).

The difference in elevation in the stages between MW and HHW amounted at Rhine to 7.4 m; in the Hase at Herzlake to 4.6 m.

The river is fordable only on the navigable reaches at MW, partly so at MW. Because of moving sand and crumbling banks, regularly used fording places are fixed.

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River flats (tailmen) are always passable for horses and motor vehicles.

River bed is fine sand.

Banks: Upper course (up to Rheins) slopes evenly, 1.5 to 2, with the banks rising up over the surrounding land. Middle course (up to Heppen). With the exception of the reach from River-km 0.0 to 72.27, where the banks are mostly steep, the valley is flat and wide. Lower course (up to Papenburg-Halte) partly falling, partly sloping (1:5 to 1:10).

Beginning at Herbrum, the river is tidal.

Depths: 0.85-1.35 m in upper course (excluding dams of 2.1-2.2 m)
2.1-3.1 m in middle course
5 --10 m in lower course, below Papenburg (beginning of Halte) and in the tidal reaches

Water-level widths: 10.6-30.0 m in the upper and middle courses; 40-120 m average in the lower course below Papenburg and in tidal reach.

Tributaries:

1. In upper course

- a. Herse (north of Muenster) water-level width at 177 6-8 m;
In some places the mill ponds are 40 m wide and 2-3 m deep.
- b. Muenstersche Aa (km 1.3)
- c. Tenningsmuehlenburg (km 5.48)
- d. Glane (km 15.88)
- e. Ems-Dattelner-Muehlenteich (km 27.34)
- f. Hamelter Bach (km 45.18)

These streams have a bank width of 2 m, mostly steep, and in reaches are cut in up to 5 m.

2. iddle course

- a. Haze (same as above)
- b. Grosse Aa (8 m wide)
- c. Salinenkanal (km 81.15) cut in 4 m deep

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3. Lower course

10 small streams, 5 of them tidal.

At Haren, the Haren-Rustenburg-Kanal branches off. It connects by means of the Nord-Sued-Kanal along the Holland border, with the Ems-Vechta-Kanal and the Holland navigation canals.

Continuous dikes exist only in the tidal region.

Dikes exist with crests above flood level.

II. Peacetime regulation and utilization of the flow.

There are no valley dams.

Control structures:

- a. In the upper course up to Rheine (River-km 46.6) there are 15 control structures without significance.
- b. In the middle course up to Hanekenfaehr (River-km 84.75) there are 4 control structures as follows:

	<u>River km</u>
1. Ems Weir (Emswehr) and Upper Lock (Obenschleuse) <u>Rheine</u> (Map No. 59, Obj. No. 125)	46.6
2. Lower Lock (Unterschleuse) <u>Rheine</u> (Map No. 59, Obj. No. 126)	47.5
3. <u>Bentlage Lock</u> (Map No. 59, Obj. No. 127)	51.7
4. Ems Weir (Emswehr) and Lock, <u>Listrup</u> (Map No. 59, Obj. No. 128)	72.0

Lowering the water stage results in preventing navigation and stopping industrial works (mills and textile factories), otherwise without significance.

- c. In the lower course the weirs serve the locks on the Dortmund-Ems-Kanal (see "Dortmund-Ems-Kanal"), as follows:

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1. Hanekenfaehr Weir (Wehr) (Map No. 58, Obj. No. 32)	Middle course River-km 84.75
2. Versen Weir (Wehr) (Map No. 58, Obj. No. 132)	Lower course River-km 10.709
3. Hilter Weir (Wehr) (Map No. 46, Obj. No. 72b)	Lower course River-km 30.89
4. Duetho Weir (Wehr) (Map No. 46, Obj. No. 67)	Lower course River-km 40.135
5. Bollingerfaehr Weir (Wehr) (Map No. 46, Obj. No. 64b)	Lower course River-km 61.1
6. Herbrun Weir (Wehr) (Map No. 46, Obj. No. 60) .	Lower course River-km 70.438

All without significance.

The Ems-Vechto-Kanal takes from the Weser R. about 2.2 m³/s of water for navigation and irrigation, which is returned to the Ems by means of the Karem-Rueterbrock-Kanal.

III. Warfare changes in the flow.

By damming the weirs and blocking the bridge openings, the valley reaches in the upper course can be flooded.

In the middle course, blocking the bridges would cause a continuous, artificial water depth of 1.8 m or more.

On the Ems Papenburg, there are no control structures, so that artificial regulation of the flow in this valley is not possible.

The summer dikes between Herbrun and Papenburg have, besides outlet structures (sluice gates and hinged gates that turn for high water stages) also back-wash gates, and can hold water back in reclaimed land for irrigation or flooding purposes. If the outlet works are completely closed, reclaimed land can be drowned out and large valley areas flooded or swamped (see General Map). The duration of the flooding depends on the weather.

In tidal sections, it is possible to keep open the sluice and tide gates during high tide and to close the gates during low tide to retain water, which will flood reclaimed land.

By the opening or blasting of a lock or a weir, a flood wave would be generated, lasting for 24 hours, which would temporarily overflow the next weir, and cause navigation to be stopped above and undanger it temporarily below.

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Ems River Basin (Sequence Downstream)

Location	Sheet No. Obj. No.	Control Structure (Name & Fl. Opening)	Navigation Effects	Remarks
1	2	3	4	5
<u>Rheine</u> 40.6 km (Kilometer- ing begins at Greven)	<u>59</u> 125	Ems Dam (Wehr)	Pool elevation Rheine Upper be lowered. (Oberachleusen) down of navigation, Navigation mill, and several the factories.	
<u>Rheine</u> 47.5 km	<u>59</u> 126	Rheine Lower	Pool runs empty. (Unterachleusen) down of navigation Navigation textile factory.	
<u>WV of Castle, Bentlage</u> 51.7 km	<u>59</u> 127	Bentlage Lower (Schleuse)	Water stage be lowered. Navigation down of navigation, in summer of a textile ery.	
<u>WV of Listrup</u> 72.0 km	<u>59</u> 128	Listrup Lock (Wehr)	Pool elevation be lowered. Listrup down navigation. Navigation	
<u>At Hase- Wehr</u> 84.75 km	<u>58</u> 32	Hase-Wehr (Wehr)	HV causes rise in stage. Navigation: At WV and HV stage falls. down navigation Hase-Wehr Canal Moppen to Glessen in the Ems River Listrup.	

*General map reference

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Ems River
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Ems River Basis (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. (Adj. No.)	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	b. Dam	a. Full Closure	
			b. Pool Width	e. Tailwater Elev.	b. Dam	c. Bridgeway	b. Full Opening	
			c. Pool Depth				c. Associated Results	
1	2	3	4		5		6	7
Rheine 46.6 km (Kilometer- ing begins at Cuxhaven)	<u>59</u> 125	Ems Dam (Wehr) and Rheine Upper Lock (Oberschleuse) Navigation	a. 8.88 km b. 47 m c. 3.7 m d. 20.54 m/H e. 26.78 m/H		a. Masonry lock chamber with wooden witer gate. Chamber length 31.3 m; chamber and gate width 5.46 m. b. Masonry fixed dam (wehr) with iron flood gate 7.4 m clear width and 45 m long.		b. Upstream: Pool elevation would be lowered. c. Shut down of navigation, one mill, and several textile factories.	
Rheine 47.5 km	<u>59</u> 126	Rheine Lower Lock (Unterschleuse) Navigation	a. 0.840 km b. 14 m c. 1.4 m d. 28.78 m/H e. 26.24 m/H		a. Masonry lock chamber with wooden witer gates. Chamber length 31.3 m; chamber and gate width 5.46 m.		b. Upstream: Pool runs empty. c. Shutdown of navigation and one textile factory.	
NW of Castle, Rastland 51.7 km	<u>59</u> 127	Beetlinge Lock (Schleuse) Navigation	a. 4.15 km b. 21 m c. 1.2 m d. 25.8 m/H e. 24.98 m/H		a. Masonry lock chamber with wooden witer gate. Chamber length 31.3 m; chamber and gate width 5.46 m. c. Walkway.		b. Upstream: Water stage would be lowered. c. Shutdown of navigation, and in summer of a textile factory.	
NW of Lidstrup 72.8 km	<u>59</u> 128	Ems Dam (Emwehr) and Lock (Schleuse) Lidstrup Navigation	a. 20.21 km b. 45 m c. 1.20 m d. 24.19 m/H e. 21.97 m/H		a. Masonry lock chamber with wooden witer gate. Chamber length 29 m; chamber and gate width 5.4 m. b. Masonry fixed dam (wehr) with iron, waste-gate and gravel card (Grisselstein- ter). Length 64 m, waste- gate width 12.8 m.		b. Upstream: Pool elevation would be lowered. c. Shutdown navigation.	
At Hase- mühlbach 84.75 km	<u>56</u> 32	Hasebühlbach Dam (Wehr) Navigation	a. 2.4 km in Dortmund-Ems Canal 12.4 km in Ems River b. 42 m c. 3.14 m d. 21.57 m/H e. 18.16 m/H		b. Fixed dam (wehr) with waste channel. Dam length 68.5 m. Waste channel length 12.6 m. Dam con- structed of masonry, waste channel of (Lössleender) and iron valves. Right bank contains a fish ladder.		a. Upstream: HW causes small rise in stage. b. Upstream: At HW and NW water stage falls. c. Shut down navigation in Dortmund-Ems Canal from Neppen to Giesen and in the Ems River to Lidstrup.	

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Elbe River
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Elbe River Pool (Sequence Downstream)							
Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	DESCRIPTION OF CONTROL STRUCTURES				
			Pool Data		Lock & Dam Data		
			a. Backwater Extent	d. Headwater Elev.	a. Lock	Operation Effects a. Full Closure b. Full Opening c. Associated Results	
			b. Pool width	e. Tailwater Elev.	b. Dam		
			c. Pool Depth		c. Bridgeway	Remarks	
W of Veersa 19.709 km (dike- metering begins at Huppen	50 192	Veersa Dam (Wehr) Navigation	a. 9.4 km b. 42 m c. 2.72 m d. 10.40 m/KM e. 8.20 m/KM		b. Needle weir. Fish ladder on left bank. length 50.6 m. The locks are located at Huesel.	b. <u>Upstream</u> : Pool from Huesel to Huppen runs empty. <u>Downstream</u> : At sudden opening by blasting, a 1.5 m high wave would be produced, which would run out in about 12 hours.	
W of Hilter 30.89 km	46 72a	Hilter Dam (Wehr) Navigation	a. 11.0 km b. 42 m c. 2.64 m d. 7.50 m/KM e. 5.30 m/KM		b. Needle weir 50.6 m long. Fish ladder on right bank.	a. Without significance. b. <u>Upstream</u> : Pool from Hilter to Huesel runs empty. <u>Downstream</u> : At sudden opening by blasting, a 1.5 m high wave would be produced, which would run out in about 12 hours. c. At low and middle flow navigation would be stopped, endangering vessels in the pool between Hilter and Duetho. Insig- nificant local inundation.	
W of Lathen 40.135 km	46 87	Duetho Dam (Wehr) Navigation	a. 9.3 km b. 30 m c. 2.64 m d. 6.00 m/KM e. 4.05 m/KM		b. Needle weir 50.6 m long. Fish ladder on right bank.	a.-c. Same as above. Endanger- ing the vessels in the pool between Duetho and Dellinger- fuehr.	
At Huesel 41.1 km	46 88b	Dellingerfuehr Dam (Wehr) Navigation	a. 11 km b. 42 m c. 2.51 m d. 3.80 m/KM e. 2.00 m/KM		b. Needle weir 50.6 m long.	a.-c. Same as above. Endanger- ing vessels from Dellinger- fuehr to Hartrun.	

*General map reference

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Red River Basin (includes Dryden)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Affected a. Pool Cleaning b. Pool Opening Associated Results	Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	b. Dam		
1	2	3	b. Pool width	e. Tailwater Elev.	c. Bridgeway		6	7
55 of Barbrum 70.430 km	46 60	Barbrum Dam (Sear) Navigation	a. 6.6 km b. 42 m c. 2.75 m d. 2.00 m/yr e. 0.40 m/yr		b. Sluice dam, 6 openings 6.5 m each. Iron gate panels, 2.5 m high, upper edge at 2.00 m/yr with movable flap at 2.30 m/yr. Fish ladder on right bank.		without significance. c. Without Pool from Barbrum to Delta ar- face runs out to free flowing river stage. Downstream: At sudden opening by blasting, a wave would be created. The wave height and duration would depend on high and low tide. c. Same as above. En- dangering vessels in the pool from Barbrum to Papenburg.	The dam is located in the tidal area. Normal stage is 2.00 m/yr. Average for the years 1901-1925. <u>Downstream High tide</u> MW 0.61 m MW 1.79 m MW 3.28 m <u>Low tide</u> MW 0.11 m MW 1.24 m MW 3.21 m MW 0.80 m

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DORTMUND-EMS CANAL

I. Description of the Watercourse.

Zero point of the kilometering is at Dortmund.

Up to northwest of Glessen (Canal-km 138.3) the canal is artificially built up; for a length of about 17 km above Hanekenfastr Weir (River-km 84.75, Map No. 58, Obj. No. 32) water impounded on the Ems R. is used; after 26 km Seitenkanal (built up "Haneken Kanal") then flows to River-km 165.8 into Hase R. and for 600 m (from above the mouth), together with this tributary, into the Ems R. From Meppen to the mouth in the Ems river harbor (River-km 269.1), the Ems R. is built up to the Dortmund-Eme Canal.

On the canalised Ems R. between Meppen and Herbrum, there are locks in the canal cut-offs adjacent to weirs (Wehre) on the loops of the Ems R. channel.

From Dortmund up to the branching of the Ems-Weser Canal (Mittelland-Kanal) at km 108.35, only 2 hydraulic structures exist:

Map No.	Obj. No.	Name	Km.
1) 82b	334	Henrichsburg Boat Elevator (Schiffhebewerk)	15.3
82b	335b	Henrichsburg High-lift Lock (Schachtschleuse)	14.95
2) 71	146	Munster i.W. Locks (3) (Schleusen-gruppe)	71.3-5

From Bevergern up to the junction with the Ems R. at Canal km 165.6, there are 9 hydraulic structures:

Map No.	Obj. No.	Name	Km.
3) 99	227	Bevergern Barge Lock (Schleppzugschleuse)	109.3
4) 99	220	Rodde Barge Lock	112.5
5) 99	208	Altenrheine Barge Lock	117.9
6) 99	252	Vanheus Barge Lock	126.6
7) 99	195	Hessolte Barge Lock	134.5
8) 58	33a	Glessen Barge Lock	137.9
9) 58	33b	Glessen Small Lock (Kleine Schleuse)	138.0
10) 58	20	Varloh Old & New Locks	158.2
11) 58	18	Teglingen Old & New Locks	164.0
12) 58	16	Meppen Old & New Locks	165.6

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Besides these, there are 7 locks in the First Dortmund-Ems Canal in which both old locks:

	Map No.	Obj. No.	Name	Km.
13)	59	230	Borghoevede Small Lock	103.6
14)	59	224	Bevern Small Lock	109.7

are combined into one hydraulic lift, and the 5 locks of the old Haneken-Kanal with the old Geeste Lock (Map No. 58, Obj. No. 21) and Neppen Double Lock (Koppelschleuse) (Map No. 58, Obj. No. 116) are both outside the navigable waterway.

The canalized Ems has 5 hydraulic lifts, with 70 m total drop, as follows:

	Map No.	Obj. No.	Name	Km.
15)	46	75a	Huentel Old Lock	175.6
16)	46	75b	Huentel New Lock	175.6
17)	46	72a	Hilter Lock	187.5
18)	46	67	Dueth Lock	196.7
19)	46	64	Bollingerfähr Lock	207.7
20)	46	60	Herbrum Lock	214.3

In addition there are two locks at Hanekenfähr:

	Map No.	Obj. No.	Name	Km.
21)	48	32	Hanekenfähr New Guard Lock (Sperrschleuse)	140.4
22)	58	123	Hanekenfähr Old Guard Lock	139.9

The Oldersum-Emsen Side Canal (Seitenkanal) (Canal-Km 260.2-269.0) is 31 m wide and 2.5 m deep and has 2 control structures:

	Map No.	Obj. No.	Name	Km.
23)	31b	81a	Oldersum Sea Lock (Seeschleuse)	-
24)	31a	195	Borsum Sea Lock	-

At full opening of the Borsum Sea Lock (Seeschleuse), the harbor water flows in the canal; at full opening of the Oldersum Sea Lock, high and low tide can pass unhindered and over flow the inland, resulting in damage to cultivated fields.

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Between Dortmund and Bergeshoevede (in 1937) the canal was being widened and deepened and one bank or the other reinforced by sheet piling, 4 m above the river bed, and in 1937, the following reaches had been reinforced; km 1.46-9.3 (left bank), km 15.3-21.05 (both banks), and km 91.8-101.5 (left bank). Further information on the structures or the sheet piling can be found on the general map.

Second route: km 21.06-30.8 at Olfen, km 49.4-50.6 at Senden, km 55.6-56.9 at Amelsbueren and km 108.6-109.7 from Bergeshoevede Lock (Schleuse) up to Bergeshoevede Small Lock (kleine Schleuse).

Weak currents exist only at locks.

The canal is navigable, with exception of infrequent ice blocking, for vessels of 67 m length, 8.2 m wide, and draft of 2.0 m (750 tons). After enlargement is complete: length 80 m, width 9.5 m, draft 2.5 m (1500 tons).

Canal bed, consists of sand or loam; some places of clay or, at Riesenbeck (km 105.7-106.4), of limestone.

Banks are sloped, protected by stone packing or filling, except on reaches with sheet piling (see general map). In limestone reaches, the banks are 9 m high.

Tow-paths are continuous on both sides.

Valley flats (Talaue): In the Dortmund area and further to Datteln, the valley is mostly industrialized, otherwise the land is agricultural. They are passable for horses and motor vehicles. There are many willow trees and ditches.

The streams crossing the canal are :

- 1) at km 19.8 - Gehlmuehlenbach, cut in deeply
- 2) at km 19.6 - Mouth of the Datteln-Hamm Canal
- 3) at km 21.3 - Mouth of the Wesel-Datteln Canal
- 4) Northward from there, the canal crosses over the Lippe Valley by means of a high earth dam and canal bridge, the same way over the Stever Valley. At this place, a second route (Fahrt) about 10 km long is being built at Olfen. Each of both routes can be shut off by guard gates on the upper and lower ends.
- 5) at km 43.4 - Kleuterbach
- 6) at km 43.6 - Nonnenbach
- 7) at km 46.2 - Stever (again)
- 8) at km 50.9 - Offenbach
- 9) at km 58.4 - Götterbach
- 10) at km 87 - below Münster the canal crosses the Ems Valley in 2 routes, by dam and by canal bridge
- 11) at km 15.6 - a side canal to Horne branches off

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II. Peacetime Regulation and Utilization of the Flow.

To replace the water lost by locks, seepage, and evaporation, replenishing of the canal is necessary.

The level pool 0.0-15.3 is fed out of the lower water through a pumping plant located on the side of the high-lift lock and the ship lock at Henrichsburg. Excess water after intense rainfall goes near the boat elevator (Schiffshoebwerk) through a relief structure which may be used to empty the pool at the same time into the lower level pool.

The level pool 15.3-71.5 is fed through the Datteln-Hamm-Kanal, which, in turn, receives water out of the Lippe at Hamm. In addition the pool is fed:

- 1) through a pumping plant at the Munster Lock (km 71.5) out of the lower water, which, in turn, is fed from the Weser R. from a pumping plant in Minden
- 2) through a pumping plant on the Wesel-Datteln Kanal from the Rhein R.
- 3) through a pumping plant at Olfen (km 23.6) from the Lippe R.

Water consumption of this pool is 10-12 m³/s.

The following relief structures serve for lowering the water stages:

- 1) Safety gate at the canal bridge over the Ems R. (km 78.9)
- 2) At the canal bridge over the Ems (second way)
- 3) Dam (Wehr) on the Ems R.

The following quantities of water out of the canal are delivered:

- 1) about 4.4 mil. m³ per year for the drinking water supply plant for the city of Munster, I.W.
- 2) 25,000 m³ per day for the city electric works of Munster, I.W. for cooling purposes
- 3) 70,000 m³ per year to "Vereinigte Asbest Dances-Wetzlar," Dortmund, for cooling and treatment purposes
- 4) 500,000 m³ per year to Dortmund "Mill Works (Muehlenwerke)" A.G. for cooling

The west dam of the side canal between Hanekensfahr and Meppen has outlets which serve for the irrigation of the meadows and lowlands between the side canal (Seitenkanal) and the Ems R. (inundation possible) and for feeding of 500 Morgen (1 Morgen equals 2.12 acres) of fish ponds.

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GENERAL
STATEMENT

Water Regulator

- 1) Outlet on the right bank of the river for the irrigation of the district (irrigation canal).
- 2) Outlet on the right bank near the lock (with high water, the "Barrage" is closed, the Ahlen channel (Ahlen Canal) is used).
- 3) On the right bank of the lock, an inlet for high water in the case of high water (possible).

III. Water Regulator in the Lock

The water regulator is a structure of reinforced concrete, the water level in the lock gates. The structure is located in the flow through the lock gates. The location of the lock gates is shown in the plan view.

Rules for operating

- 1) Closing the water regulator in the lock gates.
- 2) Opening of the water regulator in the lock gates.

Closure of the water regulator in the lock gates.

The operating time, including the time taken to close the emergency gate, is shortening of the operating time channels on the lock in the water canal, and Lock VII of the canal port, so that as they are opening the lock gates in the water stage is possible to gates.

The lock navigation is in Section II (a).

The lock navigation is in Section II (a) and the water level is possible to shipping, to the lock.

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Canal (Sequence Downstream)

DEVELOPMENT INFORMATION
DESCRIPTION OF CONTROL STRUCTURES

Canal No.	Control Structure (Name & Purpose)	Pool Data			Lock & Dam Data		Operative	
		a. Backwater Extent	d. Headwater Elev.	e. Tailwater Elev.	a. Lock	b. Dam	a. Full C/B	b. Full Op.
2	3	b. Pool Width	c. Pool Depth		c. Bridgeway		c. Associat	
622 798	Hearichsburg High- lift Lock (Schacht- schleuse) Navigation in canal pool km 0.0-14.95	a. 14.950 km b. 30 m c. max. 3 m d. 69.84 m/20 e. 56.15 m/20			a. Masonry lock with water- saving basin. Chamber length 95 m. Chamber and gate width 10 m. c. Roadway 7.5 m (heavy vehicles). Walkway 2x1.5 m. Load-bearing roller, 24 ton.		a. None b. Unusable run of (km 13.4 to safe 1.754 m km 0.0. Revised operating ately would be signifi- water's c. At a) No routed richest ter (Se At b) De Verke ing on	
	Hearichsburg Boat Elevator (Schiff- schleuse) Navigation in the canal pool km 0.0-15.3	a. 15.3 km b. 30 m c. max. 3 m d. 69.89 m/20 e. 56.15 m/20			a. Dam. 1. m. with 5 floats. Steel construction. Trough length 68 m. Trough and gate width 8.6 m. c. Roadway 4 m (heavy vehicles) Walkway 2x0.4 m.		a. Withou b.-c. Sar (from Navie- High- (Sch)	

General map reference

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Dortmund-Hann Canal
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DESCRIPTION OF CONTROL STRUCTURES

Pool Data		Lock & Dam Data	Operation Effects	Remarks
a. Backwater Extent	d. Headwater Elev.	a. Lock	a. Full Closure	
b. Pool Width	e. Tailwater Elev.	b. Dam	b. Full Opening	
c. Pool Depth		c. Bridgeway	c. Associated Results	
4		5	6	7
a. 56.3 km on the Dortmund-Hann canal 11 km on the Herne Branch Canal 40 km on the Lippe Canal. Datteln-Hann. d. 56.15 m ² /s e. 50 m ² /s		a. Masonry storage lock (Sparschlusse). Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 3.5 m (light vehicles). Walkway 2x0.5 m. Concentrated load 10 tons, 400 kg/m ² .		a. Without influence. b. <u>Upstream</u> : Canal pool runs empty up to safety gate at km 29.497 or 21.585. Failure of the safety gate would empty pool up to km 15.3, including the branch canal to Herne & Lippe Canals. <u>Downstream</u> : At sudden opening of the gate, moderately large flood wave would be created. The effect could be increased by simultaneous opening of locks 1 and 3.
) Same as Lock 2 Münster		a. Masonry, long-low bar (Schleppzug-Sparschlusse) lock with water-storage basin. Chamber length 225 m. Chamber and gate width 12 m. c. Roadway 3.5 m (light vehicles). Walkway 2x0.5 m. Class II, 10-ton, 400 kg/m ² concentrated load.		a.-c. Same as Lock 2 Münster. <u>Destruction</u> : Same as Lock 2 Münster.
) Same as above		a. Masonry, water-storage basin. Chamber length 67 m. Chamber and gate width 8.6 m. c. Roadway and walkway same as above.		a.-c. Same as above. <u>Destruction</u> : Same as Lock 2 Münster.

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Dortmund-Ruhr Canal
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Dortmund-Ruhr Canal (Seversche Donaustrasse)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects		Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	b. Dam	a. Full Closure	b. Full Opening	
			b. Pool Width	e. Tailwater Elev.	c. Bridgeway		c. Associated Results		
			c. Pool Depth						
Bergeschevede 100.6 km	59 230	Bergeschevede Small Lock (Kleine Schleuse) Navigation	a. 37.1 km b. 174.4 km in Ruhr-Weser Canal c. 10-34.25 m d. max. 3-3.2 m e. 50.0 m/ft f. 45.7 m/ft		a. Masonry lock chamber with iron mitre gate. Chamber length 67 m. Chamber and gate width 8.6 m. c. Roadway 4.5 m. Class III, 10 tons.		a. Little b. Canal bed runs dry in entire reach upstream when the guard gates are not locked. Downstream, by sudden opening (blasting), a strong wave is created. c. The Bevergern Little Lock (Kleine Schleuse) and the side dams of the canal would be flooded by the closure of the above gates. Bevergern Lands (Wiederung) would be flooded and navigation stopped.		<u>Destruction:</u> Navigation to Hamm and Enden would stop. Col. 6c. In case of war, a needle valve should be installed.
N of Bevergern 109.3 km	59 227	Bevergern Long-view Barge Lock (Schleppschleuse) Navigation	a. 37.0 km b. 174.4 km in Ruhr-Weser Canal c. 10-34.25 m d. max. 3-3.2 m e. 50.0 m/ft f. 41.7 m/ft		a. Masonry, water-storage basin. Upper gate: iron hinge gate. Lower gate: iron mitre gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 5 m (heavy vehicles) Walkway 2x1 m. Class III, 24 tons.		a.-c. Same as above. to c. Flooding of Rodde Lock and the canal side dams (Seitendämme).		<u>Destruction:</u> Same as above.
N of Bevergern 109.7 km	59 226	Bevergern Small Lock (Kleine Schleuse) Navigation	a. 3.03 km b. 30 m c. max. 3 m d. 45.7 m/ft e. 41.7 m/ft		a. Masonry lock chamber with iron mitre gate. Chamber length 67 m. Chamber and gate width 8.6 m. c. Roadway 1.5 m. (pedestrian) Schrambord 2x0.35 m.		a. Conditions unchanged. b. Canal bed fails. c. Rodde Lock and Gats would be flooded.		<u>Destruction:</u> Navigation at open between Bergeschevede and Enden. to 6c. In event of war, install needle valve.

*General map reference

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Dortmund-Rine Canal
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Dortmund-Rine Canal (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	b. Dam	a. Full Closure	
			b. Pool Width	e. Tailwater Elev.	b. Dam	c. Bridgeway	b. Full Opening	
			c. Pool Depth				c. Associated Results	
N of Rortmunde 112.5 km	56 220	Redde Long-tow Barge Lock (Schleppzug- schleuse) Navigation	a. 3.002 km b. 30 m c. max. 3 m d. 41.70 m/KM e. 37.90 m/KM		a. Masonry lock chamber. Upper gate: iron hinge gate. Lower gate: iron miter gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 4.30 m (heavy vehicles). Class III, 10 tons.		a. None. b. By blasting the gate in the lower gate the Altem- heine Lock would be flood- ed. c. For defense, needle valve should be installed.	
Altemheine 117.9 km	59 208	Altemheine Long-tow Barge Lock (Schleppzugschleuse) Navigation	a. 5.176 km b. 30 m c. max. 3 m d. 37.90 m/KM e. 34.30 m/KM		a. Masonry lock chamber. Upper gate: iron hinge gate. Lower gate: iron miter gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 4.11 m (heavy vehicles). Class III, 10 tons.		a.-c. Same as above. Floods Venhaus Lock.	
About 3 km NW of Venhaus 116.8 km	56 204	Venhaus Long-tow Barge Lock (Schleppzugschleuse) Navigation	a. 8.551 km b. 30 m c. max. 3 m d. 34.30 m/KM e. 30.80 m/KM		a. Masonry lock, chamber. Upper gate: iron hinge gate. Lower gate: iron miter gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 4.11 m (heavy vehicles). Class III, 10 tons.		a.-c. Same as above. Floods Hoeselke Lock.	
About 2 km NW of Hoeselke 134.5 km	59 195	Hoeselke Long-tow Barge Lock (Schleppzugschleuse) Navigation	a. 7.628 km b. 30 m c. max. 3 m d. 30.80 m/KM e. 27.44 m/KM		a. Masonry lock chamber. Upper gate: iron hinge gate. Lower gate: iron miter gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 4.11 m (heavy vehicles). Class III, 10 tons.		a.-c. Same as above Floods Gleesen Lock.	

*General map reference

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Dortmund-Ems Canal
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Dortmund-Ems Canal (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects		Remarks
			a. Backwater Extent b. Pool Width c. Pool Depth	d. Headwater Elev. e. Tailwater Elev.	a. Lock b. Dam c. Bridgeway	a. Full Closure b. Full Opening c. Associated Results			
1	2	3	4		5		6		7
NE of Elbergen 137.9 km	58 33a	Gleesen Long-tow Barge Lock (Schleppzugschleuse) Navigation	a. 3.187 km b. 30 m c. max. 3 m d. 27.40 m/HW e. 21.57 m/HW		a. Masonry lock chamber. Upper gate: iron hinge gate. Lower gate: iron miter gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 4.15 m (heavy vehicles). Class III, 10 tons.		a. None b. Downstream: Blasting of the gate creates a wave which runs out in the Ems R. c. Navigation would stop. Ems River would rise. Needle valves should be installed.	Navigation can be routed through Gleesen small lock (Kleine Schleuse).	
NE of Elbergen 138.0 km	58 33b	Gleesen small lock (Kleine Schleuse) Navigation	a. 3.416 km b. 30 m c. max. 3 m d. 27.40 m/HW e. 21.57 m/HW		a. Masonry, storage chamber, with iron miter gates. Chamber length 67 m. Chamber and gate width 8.6 m. c. Roadway 5.34 m (heavy vehicles). Class III, 10 tons.		a.-c. Same as above.	Same as above.	
At Hankefähr- wehr 139.96 km	58 125	Hankefähr Old Guard-Lock (Alte Sperrschleuse) Navigation	a. 2.4 km 12.4 km in the Ems River b. 42 m c. max. 3.14 m d. 21.57-23.45 m/HW e. 21.57 m/HW		a. Lock with masonry head and sloping side walls. Chamber length 45 m. width 6.1 m. Gate width 5.96 c. Walkway 4 m.		a. Little significance b. At NW and MW, none. At HW in the Ems River, the pool from Hankefähr to Verloh is raised. c. At HW in the Ems River and blasting of the gates, a wave about 1 m high would be created, endan- gering navigation, im- pounding some land, and may cause some dams to break.		
At Hankefähr- wehr 140.4 km	58 32	Hankefähr New Guard-Lock (Neue Sperrschleuse) Navigation	Same as above		a. Masonry head and sloping side walls. Chamber length 165 m. Chamber and gate width 10 m. c. Walkway 1.8 m.		a.-c. Same as above.		

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Dortmund-Ems Canal
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Dortmund-Ems Canal (Sequence Downstream)			DESCRIPTION OF CONTROL STRUCTURES					
Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects	Remarks
			a. Backwater extent	d. Headwater Elev.	a. Lock	a. Full Closure		
			b. Pool width	e. Tailwater Elev.	b. Dam	b. Full Opening		
			c. Pool Depth		c. Bridgeway	c. Associated Results		
1	2	3	4		5	6	7	
At Quaste RR Station 155.42 km	58 21	Gesote Old Lock (Alte Schleuse) Formerly for navigation of the Ems River. Not in operation.	a. 17.8 km b. 30 m c. max. 2.5 m d. 21.57 m/ft e. 19.21 m/ft		a. Masonry storage lock. Shut off at upper head by a wall. Chamber length 28.5 m. Chamber and gate width 5.96 m. c. Roadway 4.74 m.	a.-c. Same as above	Lock is not operative for navigation.	
E of Varloh 158.2 km	58 20	Varloh New Lock (Neue Schleuse) Navigation	a. 17.8 km b. 42 m c. max. 2.5 m d. 21.57 m/ft e. 17.90 m/ft		a. Masonry lock. Iron miter gates. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 4.74 m.	a.-b. Same as above c. Blasting of the gates will produce a wave 2.5 m high, endangering navigation and flooding adjacent land.		
E of Varloh 158.2 km	58 20	Varloh Old Lock (Alte Schleuse) Navigation	a. 2.8 km b. 18 m c. max. 2 m d. 19.21 m/ft e. 17.90 m/ft		a. Masonry lock with upper head shut off by a wall. Chamber length 28.5 m. Chamber and gate width 5.96 m. c. Roadway 4.74 m.	a.-c. No special significance.	Lock is not operative for navigation.	
E of Haren 164.6 km	58 18	Teglingen New Lock (Neue Schleuse) Navigation	a. 5.7 km b. 30 m c. max. 2.5 m d. 17.90 m/ft e. 14.60 m/ft		a. Masonry lock with iron miter gates. Chamber length 165 m, width 10 m. Gate width 10 m. c. Roadway 2.1 m Walkway 2x0.4 m.	a.-b. Same as above. c. By blasting, a wave 2 m high would be created, endangering navigation, flooding Neptun Lock and the old Keppel Lock (Keppelschleuse) in Old Haren Canal. Danger of dam breaking.		

*General map reference

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Dortmund-Ems Canal
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Dortmund-Ems Canal (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data			Lock & Dam Data		Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	e. Tailwater Elev.	a. Lock	b. Dam		
			b. Pool Width			c. Bridgeover		c. Associated Results	
			c. Pool Depth						
At Haren 164.0 km	58 18	Teglingen Old Lock (Alte Schleuse) Navigation	a. 5.7 km b. 30 m c. max. 2.5 m d. 17.90 m/HN e. 14.60 m/HN			a. Masonry lock with wooden miter gates. Chamber length 28.5 m. Chamber and gate width 5.96 m c. Roadway 2.15 m Walkway 2x0.4 m		a.-b. Same as Hasckenfaher Guard Lock (Sperrschleuse) (km 139.96) and Teglingen New Lock (Neue Schleuse). c. Blasting will create wave 2 m high, endangering navi- gation, flooding Huppen Lock, and the old Double- Lock (Koppelschleuse) in the Old Hascken Canal. Danger of dams breaking below Teglingen.	
At Haren 165.5 km	58 16	Huppen Old Lock (Alte Schleuse) Navigation	a. 1.7 km b. 30 m c. max. 2.5 m d. 14.70 m/HN e. 10.40 m/HN			a. Masonry lock. Upper end, iron hinge gate. Lower end, iron miter gate. Chamber length 100 m. Chamber and gate width 10 m. c. Roadway 2 m Walkway 2x0.3 m		a.-b. No significance. c. Blasting will cause the pool from Huppen to Teglingen to run dry. Navigation would stop.	
At Haren 165.6 km	58 16	Huppen New Lock (Neue Schleuse) Navigation	a. 1.7 km b. 30 m c. max. 2.5 m d. 14.70 m/HN e. 10.40 m/HN			a. Masonry lock. Upper end, hinge gate. Lower end, miter gate. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 2 m Walkway 2x0.3 m		a.-c. Same as above. The discharged wave runs out in a short time in the Ems River.	
E of Haren 165.5 km	58 116	Huppen Double Lock (Koppelschleuse) Navigation	a. 1.7 km b.-c. Same as Huppen New Lock above.			a. Masonry lock 3 wood miter gates. Chamber length 2x28.5 m. Chamber and gate width 5.96 m		a.-c. Same as Huppen Lock above.	
Huseneler- Bach (at Haren) 175.6 km	66 75a	Huseneler Old Lock (Alte Schleuse) Navigation	a. 10.1 km b. 30 m c. max. 2.72-3.03 m d. 10.40 m/HN e. 7.50 m/HN			a. Masonry heads, sloped chamber walls. Chamber length 165 m. Chamber and gate width 10 m c. Roadway 3.2 m Walkway 2x0.4 m		a. No significance. b. Downstream : Blasting creates wave 2 m high, which runs out in about 12 hours. c. At HN and H, navigation stopped. Temporary and insigni- ficant local flooding.	

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Dortmund-Rene Canal
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Dortmund-Rene Canal (Soochow Dam System)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data			Lock & Dam Data		Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	e. Tailwater Elev.	a. Lock	b. Dam	c. Associated Results	
1	2	3	4	5	6	7			
Wenteler- Bach (at Maran) 175.6 km	46 79	Wentel New Lock (Neue Schleuse) Navigation	a. 10.1 km b.-c. Same as Wentel Old Lock above.			a. Masonry. Iron miter gates. Chamber length 225 m. Chamber and gate width 12 m. c. Roadway 3.2 m Walkway 2x0.4 m	a.-c. Same as Wentel Old Lock above.		
NE of Wentel 196.7 km	46 89	Duthe Lock (Schleuse) Navigation	a. 9.3 km b. 30 m c. max. 2.64 m d. 6.00 m/H e. 3.80 m/H			a. Masonry heads. Sloping chamber walls. Iron miter gates. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 3.2 m Walkway 2x0.4 m	a. No significance. b. Dynastrom : Blasting will create a wave 1.5 m high, which runs out in about 12 hours. c. Through navigation stopped. At NW and NE, endangers vessels in the pool from Duthe to Bollingerfachs. Insignificant local flooding.		
At Maran 207.7 km	46 84	Bollingerfachs Lock (Schleuse) Navigation	a. 11 km b. 30 m c. max. 2.51 m d. 3.80 m/H e. 2.00 m/H			a. Masonry heads. Sloping chamber walls. Chamber length 165 m. Chamber and gate width 10 m. c. Roadway 2 m Walkway 2x0.3 m	a.-c. Same as above.		
S of Maran 216.30 km	46 80	Maran Lock (Schleuse) Navigation	a. 6.6 km b. 30 m c. max. 2.75 m d. 2.00 m/H e. 0.15 m/H			a. Masonry heads. Sloping chamber walls. Upper end, iron miter gate. Lower end, iron, vertical-lift gate, electrically operated. Chamber length 201 m. Chamber and gate width 10 m (lower gate 12)	a. No significance. b. Dynastrom : Pool from Maran to Bollingerfachs runs empty Dynastrom : Sudden opening by blasting creates a wave of varying height depending on high or low tide. c. Through navigation stopped. Insignificant local flooding.		
S of Olderum 240.1 km	46 81a	Olderum Sealock (Seeschleuse) Navigation Drainage	a. The Rene River in tide region. d. 5.18 m/H at high tide. e. -0.95 m/H at low tide.			a. Chamber length 100 m, width 12 m. Gate width 10 m. c. Roadway 5 m	a. None b. Inundation of lowlands. c. Damage to cultivated land. Endangers wharves on inland harbor. 2 pairs of double gates for safety.		

*General map reference

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Dortmund-Ems Canal
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Dortmund-Ems Canal (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data			Lock & Dam Data		Operation Effects		Remarks
			a. Backwater Extent	d. Headwater Elev.	e. Tailwater Elev.	a. Lock	b. Dam	a. Full Closure	b. Full Opening	
1	2	3	b. Pool Width	c. Pool Depth		c. Bridgeway		c. Associated Results		7
Borsum 269.0 km	11a 195	Borsum Lock (Schleuse) Navigation	a. Area of inland harbor. 148 hectares			a. Chamber length 100 m Chamber and gate width 10 m	c. Roadway 3.6 m, single track RR bridge	a. Canal and harbor, none.	b. <u>Canal Banks</u> : Harbor water flows in canal located at sea level. <u>Harbor Banks</u> : Water stage lowered in Emsen inland harbor.	
			c. Old Inner Harbor 5 m Navigation stage 7 m Industrial Harbor 9 m New Harbor 10.5 m					c. <u>Damage</u> to cultivated land. Endangers wharves on the inland harbor. <u>Protection</u> : Install needle valves on both heads.		
			d.-e. Harbor 1.10 m/HR Canal 0.95 m/HR							

*General map reference

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EMS RIVER, LEFT-SIDE CANALS
(Linkensische Kanäle)

I. Description of the Watercourse.

The following canals belong to this system:

- a. Ems-Vechte Canal, 21.95 km long, with connecting canal to the upper Vechte R., 0.8 km long.
- b. South-North Canal, 45.60 km long.
- c. Schoeninghedorf-Hoogeveen Canal, 2.65 km long.
- d. Haren-Rueterbrook Canal, 13.50 km long.
- e. Piccardie-Coevorden Canal, 23.50 km long.
- f. Nordhorn-Almele Canal, 4.20 km long.

The canals a) - e) belong to the Ems R. Basin; canal f) belongs to the Vechte R. Basin.

a1) Ems-Vechte Canal:

Branches out of the Ems at Hanckenfaehr Lock and joins the North-South Canal at Nordhorn.

The canal, which breaks through between the Ems and Vechte watersheds, is located in the valley of these rivers between dams, otherwise the entire length is in a 7-m deep cut.

Therefore, there is a large amount of ground-water flow and slight ice formation.

Canal bottom: Sand, with stretches of clay.

Water-level width: 16 m.

Depth: 2.1 m.

2 control structures, as follows:

<u>Map No.</u>	<u>Obj. No.</u>	<u>Name</u>	<u>Canal km.</u>
58	45	Ems Lock (Emsschleuse) at Hanckenfaehr, mouth in the Dortmund-Ems Canal	0.06
58	152a	Lock at Nordhorn-Frenswegen (mouth in the lower Vechte)	21.8

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a2) Besides 1 lock in the Verbindungs Canal with the upper Vechte:

<u>Map No.</u>	<u>Obj. No.</u>	<u>Name</u>	<u>Canal km.</u>
58	129b	Nordhorn Lock	0.7

b) South-North Canal:

The canal is located between dams. In its southern reach, it flows in the Vechte and Lee valleys, crossing in the northern reach (below Lock 18, Old Piceardie, canal-km 10.9, up to a few km above the mouth in the Harsen-Ruetenbrook Canal), the large moors located along the Netherlands border.

Canal bottom: In the southern reach, sandy; in the northern reach, sandy and peat.

Water-level width: 12-16 m.

Depth: 1.9 -2.1 m.

7 control structures, as follows:

<u>Map No.</u>	<u>Obj. No.</u>	<u>Name</u>	<u>Canal km.</u>
58	61a	Lock I at Bakolde	1.1
58	62a	Lock II at Nordhorner Wiesen	3.8
58	64a	Lock III at Hohenkoerben-Wietsmarschen	7.4
58	65a	Lock IV at Alte Piceardie	10.9
58	81a	Lock V at Hobeiermoor	34.9
46	203a	Lock VI at Pohndorf	39.6
46	209a	Lock VII at Ruetenbrook-Gosebrook	44.6

c) Schooningsdorf-Hoogerveen Canal:

Canal bottom: Sandy.

Water-level width: 16 m.

Depth: 2.1 m.

1 lock as follows:

<u>Map No.</u>	<u>Obj. No.</u>	<u>Name</u>	<u>Canal km.</u>
58	153a	Guard Lock (Sperrschleuse) on the German-Netherlands border	2.7

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d) Haren-Rueterbroeker Canals

Canal bottom: Sandy.

Water-level width: 16 m.

Depth: 2.1 m.

4 control structures, as follows:

<u>Map No.</u>	<u>Obj. No.</u>	<u>Name</u>	<u>Canal km.</u>
46	208a	Lock I at Haren	0.1
46	216a	Lock II at Altharen-Brikn	6.6
46	219	Lock III at Rueterbroek (Hinter Busch)	10.9
46	223a	Guard Lock (Sperrschlouse) on the German-Netherlands border	13.4

e) Piccardie-Coovorden Canals

Canal bottom: Sandy.

Water-level width: 12 m.

Depth: 1.8 m.

4 control structures, as follows:

<u>Map No.</u>	<u>Obj. No.</u>	<u>Name</u>	<u>Canal km.</u>
58	92a	Lock I at Hoogstede-Bathorn	7.9
58	97	Lock II at Klein Ringo	18.7
58	103a	Lock III at Volzel	19.1
58	106a	Guard lock (Sperrschlouse) IV at Vorwald	22.1

f) Nordhorn-Almelo Canals

The canal is located between dams in the Vechte Valley, otherwise in cut 4 m deep.

Canal bottom: Sandy.

Water-level width: 16-17 m.

Depth: 1.8-2.1 m.

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1 control structure, as follows:

<u>Map</u> <u>No.</u>	<u>Obj.</u> <u>No.</u>	<u>Name</u>	<u>Canal</u> <u>km.</u>
58	56a	Guard lock (Sperrschleuse) at Frensdorferboer on the German- Netherlands border	0.2

II. Peacetime Regulation and Utilization of the Flow.

The Ems R. left-bank canals are fed by ground water in the cut reaches and by several inlets from the moors through the Ems at Hanckenfehr (Map No. 58, Obj. No. 45, see General map) and the Vechte (Map No. 58, Obj. No. 152, see General map) at Nordhorn.

Out of the Ems R. at Hancken Dam (Wehr Hancken) only 2.2 m³/s can be taken at NW and NE, or navigation in the Ems R. and the Dortmund-Ems Canal will be impaired. Through the Haren-Rueterbroek Canal the water flows back to the Ems R. at Haren. In the Ems-Vechte Canal and in the upper pool of the South-North Canal this is possible through both mill dams (Muehlenwehre) at Nordhorn (Map No. 58, Obj. No. 150 and 151), the lock (abschlagswehr) to the Vechte (Map No. 58, Obj. No. 152a) and Lock I on the South-North Canal (Map No. 58, Obj. No. 61). The by-pass canal (Entlastungskanal) below Lock II of the Coevorden-Pieterdie Canal (Map No. 58, Obj. No. 97) drains again into the Vechte R. at Emlichheim.

High water in the Ems and Vechte Rivers is kept out of the canals by closing the inlet locks. The Nordhorn-Almelo Canal remains entirely in the sandy area (Staubeereich) of the Vechte.

The principal canals crossing the German-Holland border have guard-locks (Sperrschleusen) to eliminate large volumes of water in accordance with treaty arrangements.

In addition to serving navigation with lock sizes (effective length 33 m., effective width 6.50 m) corresponding to vessels, the left-side canals serve to impound water for uses in the operation of 2 mills in Nordhorn, for cooling purposes by textile factories in Nordhorn, and for cleansing purposes by a potato-flour mill in Emlichheim.

III. Warfare changes in the flow.

By complete closure of both mill dams (Muehlenwehre) on the Vechte (Map No. 58, Obj. No. 150 and 151) and the lock (abschlagswehre) (Map No. 58, Obj. No. 152a) and by opening Lock I (Map No. 58, Obj. No. 61a), the entire Vechte flow will be diverted to the South-North Canal. It is also possible, at Lock I on the Ems-Vechte Canal (Map No. 58, Obj. No. 45), to take out considerably more than 2.2 m³/s from the Ems R.

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With both quantities of water, some installations in the low and slightly sloping land between Nordhorn and Geergeders (see General map) can be inundated, because the water level in the canal is considerably higher than the surrounding land, also the water depth in the individual canal pools can be raised several decimeters by running up the lock gates.

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Ems River - Left side canals
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Ems River Left Side (Linkensische) Canals (Sequence Downstream)			DESCRIPTION OF CONTROL STRUCTURES				
Location River km	Chart No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data	Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	a. Full Closure	
			b. Pool Width	e. Tailwater Elev.	b. Dam	b. Full Opening	
			c. Pool Depth		c. Bridge	c. Navigation Effects	
1	2	3	4		5	6	7
a ¹) Ems-Vechte Canal							
Backwater Lock 0.050 km	58 45	Lock (Schleuse) I Flood control feeding the left- side canals	b. 16 m c. max. 2 m d. 23.40 m/HN e. 21.52 m/HN		a. Masonry lock chamber with wooden miter gates; chamber length 40 m, width 6.0 m. Gate width 6.5 m.	a. Downstream : Blocking of the inlet from the Ems River and water defi- ciency in the 112 km-long canals. b. Downstream : Overflowing from canal dams at NV on the Ems River. c. Shut off navigation and industrial operations. Inundation.	Restriction : Lost by demolition of upper or lower gates. The pool runs empty.
Backwater 21.2 km	58 152a	Outlet dam (Ablasse- wehr) Flood control Navigation	a. 23 km b. 16 m (entire canal) c. max. 2 m d. 21.50 m/HN e. 19.25 m/HN		b. Masonry dam (Wehranlage) wooden gate sill. 19.90 m/HN width 4 m	a. Upstream : Overflowing from canal dams at in- tensive precipitation. b. Upstream : Navigation shut off entirely. Water defi- ciency in the left-side canals. c. Inundation; stopping of industrial operations.	In the upper head of the old double lock (Koppelschleuse), which is out of operation, a sluice- dam installation (Schuttschwehranlage) is located. Opening 4 m wide and 2 m high. Gate boards (Schutts- tafel) are horizontal; divided into 2 sections.
a ²) Connecting Canal (Verbindungskanal) between the Ems-Vechte Canal and the Vechte R.							
Backwater 0.7 km	58 129b	Lock (Schleuse) II Flood control against the Vechte River Navigation	a. 23 km b. 16 m c. max. 2 m d. 21.40-22.70 m/HN e. 21.50 m/HN		a. Masonry heads with iron miter gates and sloping chamber walls. Chamber length 40 m, width 6 m. Gate width 6.5 m.	a. Shutdown of navigation to Holland. b. Downstream : Flooding from canal dams at in- tensive precipitation. c. to b. Inundation.	
b) South-North Canal (Süd-Nord-Kanal)							
Backwater 1.1 km	58 61a	Lock (Schleuse) I Navigation Feeding of canal	a. 23 km b. 16 m (entire canal) c. max. 2 m d. 21.52 m/HN e. 19.25 m/HN		a. Masonry lock chamber with wooden miter gates. Chamber length 33 m, width 6 m. Gate width 6.5 m.	a. Downstream : Water defi- ciency in the 89 km in downstream canals. b. Upstream : Emptying of canal pool. Downstream : Water-stage rises and overflows the gates of Lock II. c. Interruption of navi- gation.	Restriction : The simplest, from the upper and lower gates. The chamber walls of the lock, 1:8 slope with arched tops be- tween iron forms, based on wooden piles.

*General map reference

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See River - Left side canal
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See River Left-Side (Laktesleiche) Canals (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Ch. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	b. Dam		
			b. Pool Width	e. Tailwater Elev.	c. Bridgeway		a. Full Closure	
			c. Pool Depth				b. Full Opening	
							c. Associated Results	
5 of Wistmar- schles Northern Meadow (Wienau) 3.0 km	55 62a	Lock (Schleuse) II Navigation	a. 2.7 km b.-c. Same as Lock I above. d. 19.25 m/H e. 18.0 m/H		a. Same as Lock I above. c. Iron, field-path turn-bridge (Feldweg- Drehbruecke).		a.-c. Same as above. Overflowing of the gates at Lock III.	<u>Destruction:</u> Same as Lock I above. In the Lock sill at Lee River, back-siphon is located.
Zehen- kerben Wistmar- schles 7.4 km	56 64a	Lock (Schleuse) III Navigation	a. 4.7 km b.-c. Same as Lock I above. d. 18.00 m/H e. 16.75 m/H		a. Same as Lock I above. c. Iron, field-path turn- bridge (Feldweg- Drehbruecke)		a.-c. Same as Lock I above. Overflowing the gates of Lock IV.	<u>Destruction:</u> Same as Lock I. In the lock sill a Lehner back-siphon is located. Chamber walls of the lock are the same as Lock I.
Alte Fischerei 10.9 km	56 65a	Lock (Schleuse) IV Navigation	a. 3.5 km b.-c. Same as Lock I. d. 16.75 m/H e. 16.00 m/H		a. Same as Lock I above. c. Iron, field-path turn- bridge (Feldweg- Drehbruecke)		a. Downstream water shortage. b. Upstream: Water stage lowered about 0.75 m. Downstream: Water stage rises. c. to a. Shut off through navigation. to b. In the upper pool only vessels with small draft could navi- gate.	<u>Destruction:</u> And chamber walls same as Lock I.
5 of Fahler- bach 36.9 km	56 81a	Lock (Schleuse) V Navigation	a. 31.9 km b.-c. Same as Lock I. d. 16.0 m/H e. 15.0 m/H		a. Masonry heads, slop- ing chamber walls. Chamber length 33 m, width 6.5 m		a.-c. Same as Lock I. Overflows the gates on Lock VI.	<u>Destruction:</u> Same as Lock I.
5 of Fahler- bach 39.6 km	56 203a	Lock (Schleuse) VI Navigation	b. 12 m (at Fehndorf) c. 1.8 m (max. at Fehndorf) d. 15.0 m/H e. 13.0 m/H For the entire canal, same as Lock I.		a. Same as Lock V. c. Roadway 2.5 m. Load 6 tons.		a.-c. Same as Lock I. Overflows the gates on Lock VII.	<u>Destruction:</u> Same as Lock I. The lock- installation diversion canal is 1 m diameter, with inlet gates.
Fahler- bach Gesebrock 46.6 km	56 207a	Lock (Schleuse) VII Navigation	a. 5 km b.-c. Same as Lock I. d. 13.0 m/H e. 11.4 m/H		a. Masonry lock chamber with miter gates. Chamber length 33 m. Chamber and gate width 6.5 m. c. Roadway 2.5 m. Load 4 tons.		a.-c. Same as Lock I. Overflows the gates on Lock III of the Haren- Rustenbrock Canal.	<u>Destruction:</u> Same as Lock I. Of the lock- installation diversion works (1 m in diameter), and the inlet gates.

*General map reference

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Ems River - Left side canals
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Ems River Left-Side (Linkensleche) Canals (Sequence Downstream)

DESCRIPTIONS OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data		Lock & Dam Data		Operation Effects	Remarks
			a. Backwater Extent	d. Headwater Elev.	a. Lock	b. Dam		
			b. Pool Width	e. Tailwater Elev.	c. Bridgeway		b. Full Opening	
			c. Pool Depth				c. Associated Results	
c) Schoonhede-Boezinge Canal								
Schoonhede- Boezinge	58 153a	Guard lock (Sperrschleuse) Navigation	a. 31.9 km b. 16 m c. max. 2 m d. 16.00 m/HV e. 13.50-16.44 m/HV		a. Masonry heads, double- board, sloping chamber walls, miter gates. Chamber, length 36 m. Chamber and gate width 6.5 m. c. Iron turn-bridge for vehicles.		a. Without importance. b. Depends on the water stage in adjacent canals in Holland. c.-a. Navigation to Holland shut off. b. Water can flow to Holland. Navigation possible only for vessels with small draft. Dam of sheet piling.	
d) Haren-Bathenbrucher Canal								
Haren	56 200a	Lock (Schleuse) I Guard lock (Sperr- schleuse) Navigation	a. 6.7 km b. 16 m c. max. 2.1 m d. 8.30 m/HV e. 7.70 m/HV (OV) 10.37 m/HV (HV)		a. Masonry lock chamber double-board. Chamber length 33 m. Chamber and gate width 6.5 m. c. Roadway 2.5 m Load 4 tons		a. Without importance. b. <u>Upstream</u> : At OV on the Ems River the water stage is lowered 0.6 m. c. Through navigation shut down to a. At OV the Ems River would be navigable only for vessels of small draft. At OV Ems River water leaks into the canal.	<u>Destruction</u> : Root by demolition of upper and lower gates, the lock diversion works (1 m diameter) and the inlet g.
V of Haren	56 210a	Lock (Schleuse) II (Altharen-Erika) Navigation	a. 4.1 km b. 16 m c. max. 2.1 m (Altharen) d. 10.30 m/HV e. 8.30 m/HV		a. Masonry lock chamber. Miter gates. Chamber length 33 m. Chamber and gate width 6.5 m. c. Roadway 2.5 m. Load 16 tons		a. <u>Upstream</u> : Water stage rises slowly. <u>Downstream</u> : Water shortage. b. <u>Upstream</u> : Pool runs empty. <u>Downstream</u> : Water stage rises and overflows the gates at Lock I, but not when the Ems stage is higher than HV. c. Through navigation stopped.	<u>Destruction</u> : Same as above. Lock diversion works same. Chamber walls of the lock 1:8 with arched tops between iron forms, based on piling.
Bathenbrucher	56 Hinteren Busch 219 10.9 km	Lock (Schleuse) III Navigation	a. 3.4 km b. 16 m c. max. 2.1 m (Altharen) d. 11.4 m/HV e. 10.3 m/HV		a. Masonry lock chamber. Same as above.		a. <u>Upstream</u> : Same as above. <u>Downstream</u> : Same as above. b. <u>Upstream</u> : Pool runs out to about 1 m depth. <u>Downstream</u> : Water stage rises and overflows the gates on Lock II. c. Same as above.	<u>Destruction</u> : Diver- sion works and chamber walls, same as above.

*General reference map

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Rm River - Left side canals
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Rm River Left-side (Linkensluibe) Canals (Sequence Downstream)

DESCRIPTION OF CONTROL STRUCTURES

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	Pool Data			Lock & Dam Data			Remarks
			a. Backwater Extent	d. Headwater Elev.	e. Tailwater Elev.	a. Lock	b. Dam	c. Associated Results	
1	2	3	b. Pool Width	c. Pool Depth	4	5	6	7	8
<u>Linkensluibe</u> 22.3 km	56 223a	Lock (Schleuse) IV Guard lock (Sperr- schleuse) Navigation	a. 3.4 km (backwater also extends up Holland Canal) b. 16 m c. max. 2.1 m d. 11.4-12.5 m/HV e. 11.4 m/HV			a. Masonry head, miter gates, sloping chamber walls. Chamber length 33 m. Chamber and gate width 6.5 m. c. Roadway 2.5 m. Load 6 tons		a. Without significance. b. <u>Downstream</u> : At high- est water levels the gates at Lock III are flooded. c. to a: Navigation shut down. to b: Navigation at HV endangered.	<u>Destruction</u> : Same as above. Upper head of lock has a short culvert.
<u>Richard's Conversion Canal</u>									
<u>Richard's</u> 7.9 km	50 92a	Lock (Schleuse) I Navigation	a. 31.9 km b. 16 m c. max. 2 m d. 15.00 m/HV e. 15.00 m/HV			a. Masonry lock chamber same as above. c. Iron, field-road turn-bridge.		a. <u>Upstream</u> : At HV, water stage rises. <u>Downstream</u> : Water shortage. b. <u>Upstream</u> : Water stage lowered 1 m. <u>Downstream</u> : Over- flowing of gates on Lock II.	<u>Destruction</u> : Same as above. Relief canal for HV discharges. The inflow gates to 6b. Navigation in the South-North Canal Locks IV-V also im- possible (about 1 m water depth).
<u>Julius</u> 13.7 km	50 97	Lock (Schleuse) II Navigation	a. 5.5 km b. 16 m c. max. 2 m d. 15.00 m/HV e. 15.90 m/HV			a. Masonry head, sloping chamber walls, miter gates. Chamber length 33 m. Chamber and gate width 6.5 m.		a. At HV, water stage rises. b. <u>Upstream</u> : Water stage lowered about 1.2 m. c. Navigation interrupted.	<u>Destruction</u> is easiest by demolishing the upper or lower gates. The lock drainage works at HV discharges.
<u>Volkel</u> 19.1 km	50 107a	Lock (Schleuse) III Navigation	a. 5.4 km b. 16 m c. max. 2 m d. 13.8 m/HV e. 11.5 m/HV			a. Masonry lock chamber same as above. c. Field-road turn-bridge.		a. <u>Downstream</u> : Water shortage. b. <u>Upstream</u> : Canal runs empty. <u>Downstream</u> : Water stage rises, and overflows gates on Lock IV. c. Navigation interrupted.	<u>Destruction</u> : Same as above.
1.5 km from the German- Holland Border <u>Harstad</u> 22.1 km	50 106a	Lock (Schleuse) IV Navigation	a. 3 km b. 16 m c. max. 2 m d. 11.50 m/HV e. 9.40 m/HV			a. Masonry lock chamber same as above. c. Field-road turn-bridge.		a. Without significance. b. <u>Upstream</u> : Canal runs empty. <u>Downstream</u> : Water stage rises. c. Navigation interrupted.	<u>Destruction</u> : Same as above. Chamber walls 1:6 slope, arched tops, between iron forms, based on wooden piling.

*General map reference

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Don River Left-side (Linkensiecke) Canals (Sequence Downstream)

Location River km	Sheet No. Obj. No.	Control Structure (Name & Purpose)	DESCRIPTION OF CONTROL STRUCTURES			Remarks
			Pool Data	Lock & Dam Data	Operation Effects	
1	2	3	a. Backwater Extent b. Pool Width c. Pool Depth	d. Headwater Elev. e. Tailwater Elev. a. Lock b. Dam c. Bridgeover	a. Full Closure b. Full Opening c. Associated Results	7
2) <u>Northside-Almelo Canal</u> Franser- ferhaar 0.2 km	<u>54</u> <u>56a</u>	Guard Lock (Sperr- schluse) to Holland	a. 3.5 km b. 16 m c. max. 2 m d. 21.40-22.70 m/H e. 21.65 m/H	a. Masonry lock chamber with iron mitre gates. 2-way turning. Chamber length 40 m, width 4 m. Gate width 6.5 m.	a. Shut down navigation to Holland. b. Significant only when the water drains to Holland. c. Shortage of cooling water for 2 textile factories.	<u>Destruction: Same</u> <u>as above.</u>

*General reference map